

INFLOW DESIGN FLOOD CONTROL PLAN PERIODIC 5-YEAR REVIEW

CFR 257.82

Primary Bottom Ash Pond

Flint Creek Plant
Gentry, Arkansas

October, 2021

Prepared for: Southwestern Electric Power Company

Prepared by: American Electric Power Service Corporation

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PERIODIC 5-YEAR REVIEW
CFR 257.82
FLINT CREEK PLANT
PRIMARY BOTTOM ASH POND

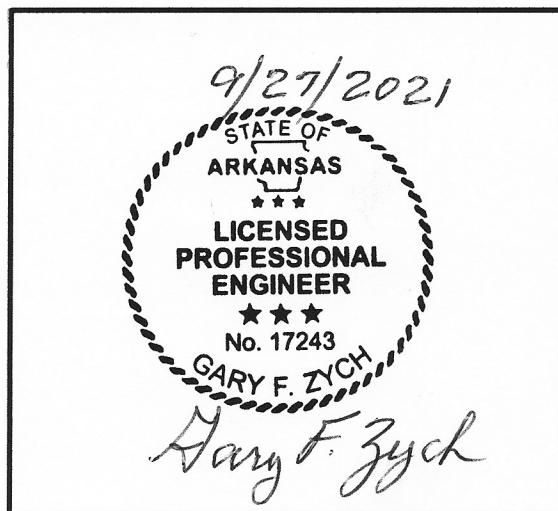
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I certify to the best of my knowledge, information, and belief that the information contained in this inflow design flood control plan meets the requirements of 40 CFR § 257.82

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Attachment A – Hydraulic Analysis of Flint Creek Power Plant Ash Ponds

1.0 OBJECTIVE

This report was prepared by AEP- Geotechnical Engineering Services (GES) section to fulfill requirements of CCR 257.82 for the hydrologic and hydraulic evaluation of CCR surface impoundments. This report is a summary of the periodic 5-year review of the initial evaluation.

2.0 DESCRIPTION OF THE CCR UNIT

The Flint Creek Power Plant is located near the City of Gentry, Benton County, Arkansas. It is owned and operated by Southwestern Electric Power Company (SWEPCO). The facility operates one surface impoundment for storing CCRs, referenced as the Primary Bottom Ash Pond.

The Primary Ash Pond dam is a cross valley dam on a tributary to the Little Flint Creek. The dam is 45 feet high and has side slopes of 3H:1V. The downstream slope is partially submerged by the Little Flint Creek Reservoir.

3.0 INFLOW DESIGN FLOOD 257.82(a)(3)

The facility is classified as a Low Hazard Potential Dam. This classification has not changed since the initial evaluation. The Inflow Design Flood is the 100-year flood.

4.0 FLOOD CONTROL PLAN 257.82(c)

All storm water runoff from the watershed drains into the reservoir created by the Primary Bottom Ash Dam. The design to safely pass the inflow design flood without overtopping the crest of the dam is based on the spillway system and surcharge flood storage capacity above the maximum operating level.

The analysis in Attachment A provides the description of the spillway system, flood storage capacity, inflow peak discharge and volume, peak discharge from the facility and maximum pool elevation.

There has not been any changes to spillway system, flood storage capacity or rainfall estimates that would change the results presented in Attachment A. The calculations show that the facility has the capacity to manage the inflow design flood, as well as larger flood events.

ATTACHMENT A



Innovative approaches
Practical results
Outstanding service

Hydraulic Analysis of Flint Creek Power Plant Ash Ponds

American Electric Power Company

Prepared by:

FREESE AND NICHOLS, INC.
4055 International Plaza, Suite 200
Fort Worth, Texas 76109
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AEP10431



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APPENDICES

Appendix A – References

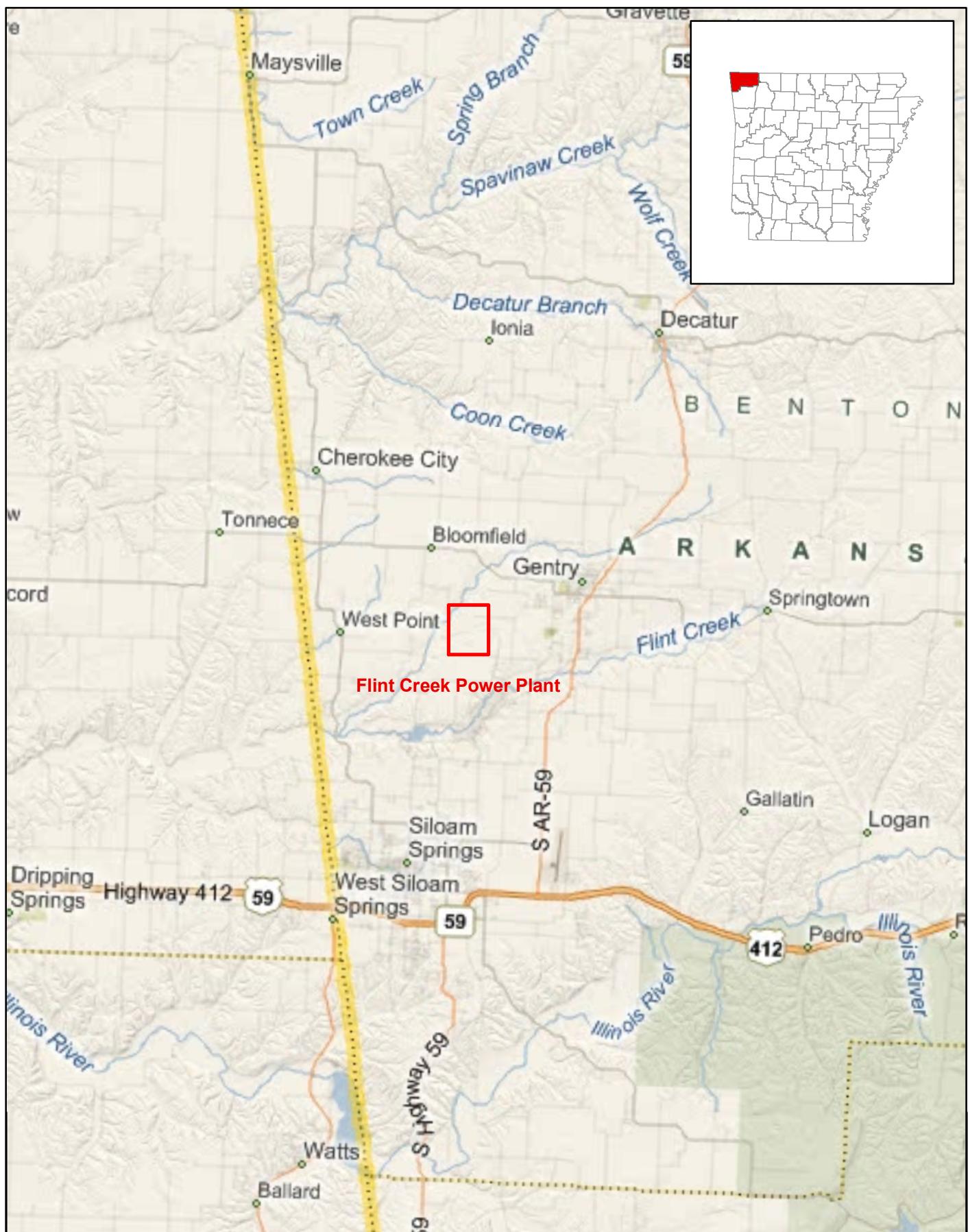
Appendix B – Discharge Rating Curve Calculations and Hydrologic Parameters

Appendix C – Pertinent Drawings

1.0 INTRODUCTION

In November of 2010, Freese and Nichols, Inc., (FNI) was retained by American Electric Power (AEP) to perform various hydrologic and hydraulic calculations to determine the hydraulic adequacy of the Primary Ash and Secondary Ash Ponds for the Flint Creek Power Plant located near Gentry, Arkansas. This report summarizes the results of the analysis for the 10-year, 25-year, 100-year, 25% PMF, 50% PMF, and 100% PMF events.

The two Ash Ponds are situated immediately south of the Flint Creek Power Plant on the east side of Little Flint Creek Reservoir. The general location of the power plant and associated reservoirs is shown in Figure 1.



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DATUM & COORDINATE SYSTEM	NAD83 STATE PLANE ARKANSAS NORTH (FT)
DATE CREATED	DECEMBER 2010
PREPARED BY	JPM



0 1.25 2.5 5 Miles

FLINT CREEK POWER PLANT ASH PONDS LOCATION MAP



**FIGURE
1**

2.0 HYDROLOGIC MODEL DEVELOPMENT

2.1 BASIN DELINEATION & CONNECTIVITY

The hydrologic model for the Flint Creek Power Plant Ash Ponds was created in HEC-HMS¹ and consisted of two total drainage basins, as shown in Figure 2. The total drainage area modeled is approximately 1.82 square miles, or 1,167 acres. One basin represents the total area that drains directly into the Primary Ash Pond, and the other represents the area that drains only to the Secondary Ash Pond. The basins were delineated from the National Elevation Dataset (NED) 10-meter resolution Digital Elevation Model (DEM).

The Primary Ash Pond is connected to the Secondary Ash Pond via a wide open channel controlled by a currently silted over concrete sill and a small weir box at a slightly lower elevation. Discharges from the Secondary Ash Pond flow into Little Flint Creek Reservoir through a similar structure. The concrete sill, however, is visible and the weir box has recently been replaced. Spillway capacities are discussed in further detail in Section 2.4.

Both the Flint Creek Power Plant and the City of Gentry Wastewater Treatment Plant discharge directly into the Primary Ash Pond. Discharges from the power plant consist of low volume wastewater and stormwater. Based on data from AEP, these discharges were assumed constant at a flow rate of 8.08 MGD, or 12.5 cfs.



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0 1,000 2,000 4,000
Feet

FLINT CREEK POWER PLANT ASH PONDS
DRAINAGE BASIN MAP



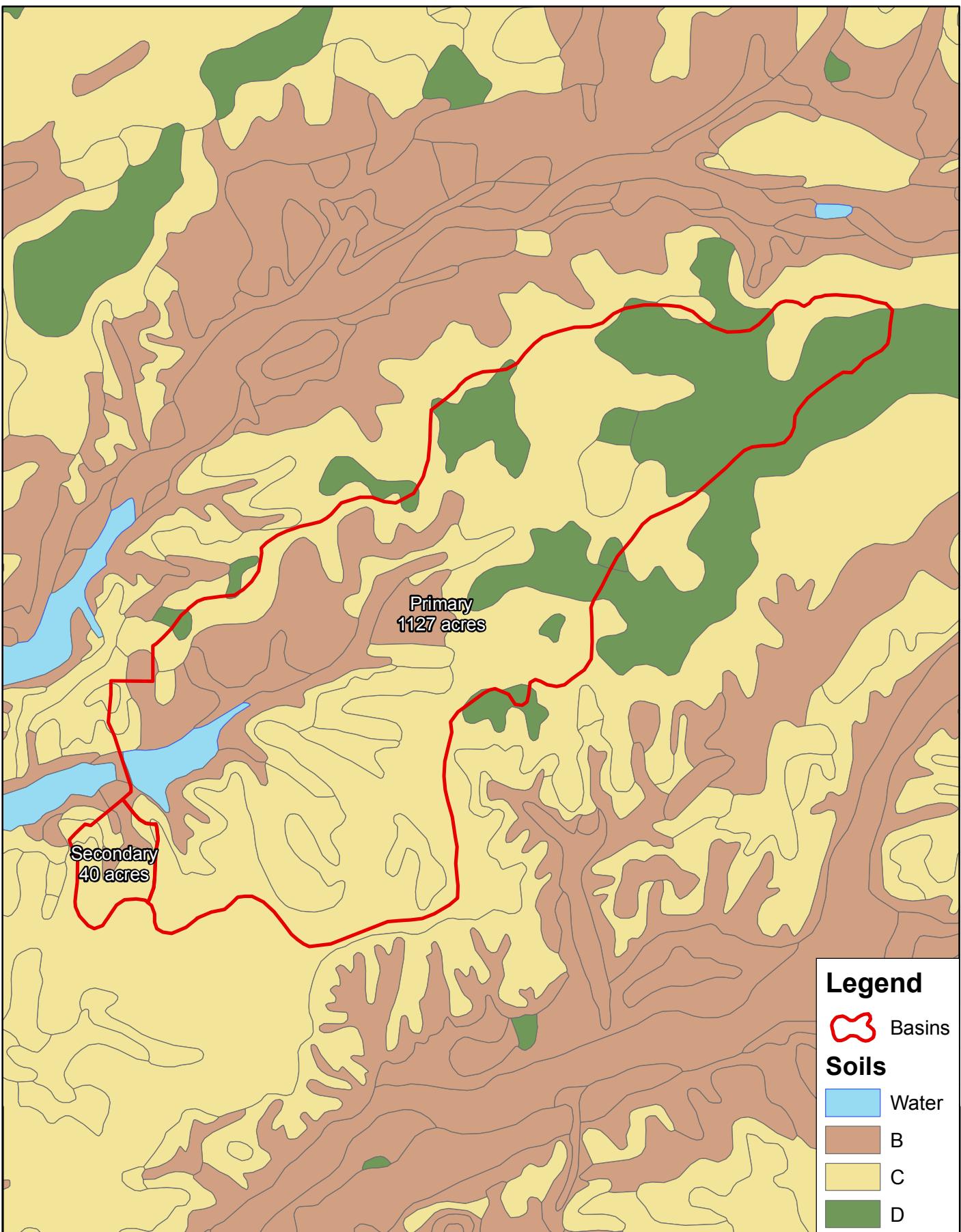
FIGURE
2

2.2 HYDROLOGIC PARAMETERS

The HEC-HMS model incorporates the NRCS Curve Number and Unit Hydrograph methods for each basin. In this model, the curve numbers were based on hydrologic soil classifications and land cover. The instantaneous runoff effect of open water surfaces was accounted for in the development of the curve numbers. The soils dataset was obtained from the NRCS Soil Survey Geographic Database² (SSURGO), and land use dataset was obtained from the USGS Seamless Data Warehouse³ in the form of the National Land Cover Dataset (NLCD) for 2001. Spatial information about soil types and land use classifications is presented in Figures 3 and 4, respectively. Table 1 provides the matrix used in determining the curve number for each basin. The curve numbers shown in Table 1 are for Antecedent Moisture Condition (AMC) II. These values were incorporated in the model for the frequency storm events, such as the 100-year storm event, and the PMP event. Typically, a higher curve number would be used to simulate a worst-case scenario with the ground fully saturated. However, because of the long duration of the PMP event and the timing of the rainfall distribution, the ground will be fully saturated prior to the peak of the storm and a higher curve number will have no significant impact on the results.

Table 1 – Curve Number Calculation Matrix

NLCD Classification		Curve Number (AMC II)					
#	Description	A	B	B/C	C	C/D	D
11	Open Water	100	100	100	100	100	100
21	Developed, Open Space	68	79	83	86	88	89
22	Developed, Low Intensity	51	68	74	79	82	84
23	Developed, Medium Intensity	77	85	88	90	91	92
24	Developed, High Intensity	89	92	93	94	95	95
31	Barren Land	77	86	89	91	93	94
41	Deciduous Forest	36	60	67	73	76	79
42	Evergreen Forest	36	60	67	73	76	79
43	Mixed Forest	36	60	67	73	76	79
52	Scrub/Shrub	35	56	63	70	74	77
71	Grassland/Herbaceous	39	61	68	74	77	80
81	Pasture/Hay	39	61	68	74	77	80
82	Cultivated Crops	67	78	82	85	87	89
90	Woody Wetlands	45	66	72	77	80	83



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0 1,000 2,000 4,000 Feet

FLINT CREEK POWER PLANT ASH PONDS
HYDROLOGIC SOIL CLASSIFICATIONS



Legend

Basins

Soils

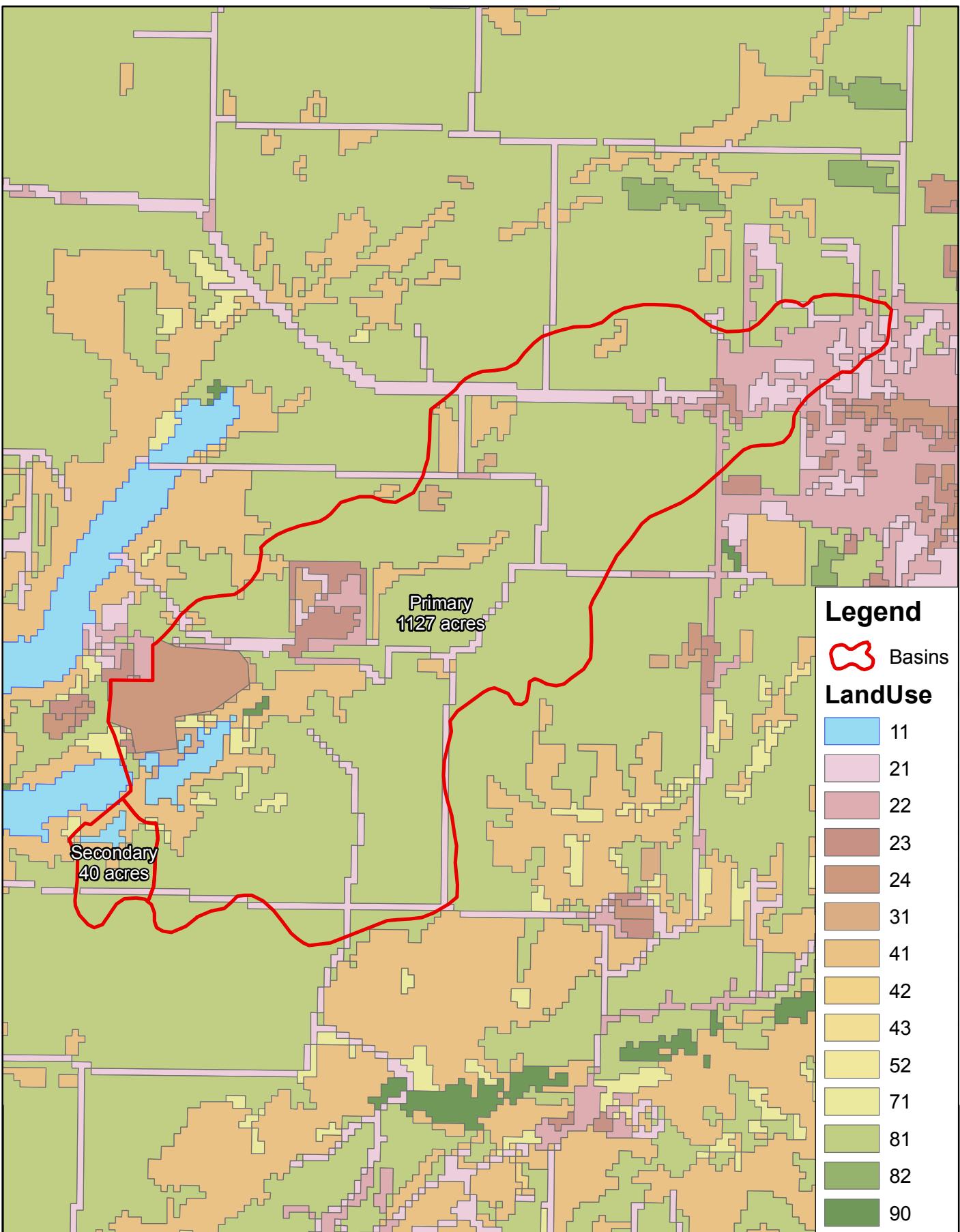
Water

B

C

D

FIGURE
3



FN PROJECT NO.	AEP10431
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DATUM & COORDINATE SYSTEM	NAD83 STATE PLANE ARKANSAS NORTH (FT)
DATE CREATED	DECEMBER 2010
PREPARED BY	JPM



0 1,000 2,000 4,000
Feet

FLINT CREEK POWER PLANT ASH PONDS

LAND COVER DATA



FIGURE
4

The only input into HEC-HMS for the NRCS Dimensionless Unit Hydrograph is a lag time, which is calculated based on basin conditions, such as hydraulic length and average slope, according to the NRCS TR-55 Method. Table 2 provides a summary of the hydrologic parameters for each basin.

Table 2 – Basin Parameters

Basin	Area (mi ²)	Lag Time (min)	Curve Number (AMC II)
Primary	1.76	46.58	76.6
Secondary	0.06	10.53	74.9

2.3 ELEVATION-STORAGE DATA

Elevation-storage data for each reservoir was obtained from a combination of two data sources. Volume calculations based on 5-foot contours were provided by AEP up to elevation 1145.0 ft-msl. The NED 10-meter DEM was utilized to calculate the available storage between this elevation and the top of dam elevation of 1155.0 ft-msl. These relationships were used in the hydrologic model for routing both frequency storm events and the PMF and are shown in Table 3 below.

Table 3 – Elevation-Storage Data

Primary		Secondary	
Elevation (ft-msl)	Storage (acre-ft)	Elevation (ft-msl)	Storage (acre-ft)
1115	0.00	1130	0.00
1120	4.59	1135	2.71
1125	23.05	1140	7.39
1130	50.92	1142.5	10.79
1135	86.32	1143	11.47
1140	133.10	1144	12.83
1144	182.55	1145	14.19
1145	186.13	1146	20.54
1146	190.47	1147	26.88
1147	195.62	1148	33.23
1148	201.66	1149	39.58
1149	208.67	1150	45.93
1150	222.59	1151	55.13
1151	263.45	1152	64.71
1152	312.36	1153	74.71
1153	363.73	1154	85.10
1154	417.40	1155	95.92
1155	473.32		

2.4 DISCHARGE RATING CURVES

Each dam has a single spillway structure with two components – a weir box acting as the principal spillway and a concrete sill acting as the emergency spillway. Information regarding the dimensions and elevations of each of these spillways was taken from a combination of original construction drawings and detailed descriptions from AEP personnel. Detailed calculations for the discharge rating curves of each spillway are included in Appendix B.

The principal spillway for the Primary Ash Pond consists of a weir box with a 4-foot wide weir with crest elevation of 1144.0 ft-msl. The weir equation used for this weir box was provided by AEP personnel. At elevation 1146.0 ft-msl, flow reaches the 228-foot long concrete sill, effectively the emergency spillway, and the weir box is assumed to be submerged, meaning flow is completely controlled by the emergency spillway. The sill is located relatively close to the flat natural grade and is currently covered with soil and light vegetation due to silting over the years. As such, the emergency spillway is modeled as a broad-crested weir, and the

discharge rating curve was developed with a steady-state HEC-RAS⁴ model. The HEC-RAS model accounts for submergence of the tailwater from the downstream lake, which will significantly restrict flow through the spillway. The discharge rating curve for the combined spillway of the Primary Ash Pond is shown in Table 4. A photograph of the spillway is shown in Figure 5.



Figure 5 – Primary Ash Pond Spillway

The principal spillway for the Secondary Ash Pond consists of a recently reconstructed weir box with a 13-foot wide weir with crest elevation of 1142.5 ft-msl. Calculations at several critical discharges were given on the construction drawings for this modification. These values were interpolated between to obtain a discharge rating curve at even one-foot increments. At elevation 1145.0 ft-msl, flow reaches the 250-foot long concrete sill, effectively the emergency spillway, and the weir box is assumed to be submerged, meaning flow is completely controlled by the emergency spillway. While the concrete sill is more defined than the one at the Primary Ash Pond, the effects of submergence were still a concern due to the flat topography and Little Flint Creek Reservoir immediately downstream. Similar to the Primary Ash Pond spillway, this spillway was modeled in HEC-RAS. The discharge rating curve for the combined spillway of the Secondary Ash Pond is shown in Table 4. A photograph of the spillway is shown in Figure 6.



Figure 6 – Secondary Ash Pond Spillway
Table 4 – Discharge Rating Curves

Primary		Secondary	
Elevation (ft-msl)	Total Discharge (cfs)	Elevation (ft-msl)	Total Discharge (cfs)
1144	0	1142.5	0
1145	13	1143	17
1146	34	1144	78
1147	305	1145	165
1148	1,071	1146	536
1149	2,208	1147	1,355
1150	3,603	1148	2,419
1151	5,133	1149	3,735
1152	6,873	1150	5,310
1153	8,816	1151	7,118
1154	10,978	1152	9,174
1155	13,325	1153	11,463
		1154	13,974
		1155	16,484

2.5 FREQUENCY MODEL RESULTS

Three frequency storm events were analyzed for the Flint Creek Ash Pond system – the 10-year, 25-year, and 100-year storm events. The hydrologic model described in the preceding sections was implemented in analyzing these events. Curve numbers were set to Antecedent Moisture Condition II, and initial abstractions were calculated automatically by HEC-HMS. These assumptions represent normal conditions, as would be expected prior to one of these storm events. The precipitation data was obtained from the National Oceanic and Atmospheric Administration's Technical Memorandum NWS HYDRO-35⁵ and Technical Paper 40.⁶ These values are presented in Table 5. Each storm event was assumed to have a duration of 24 hours.

Table 5 – Frequency Precipitation Depths

Frequency (yrs)	Precipitation (in)							
	5 min	15 min	60 min	2 hr	3 hr	6 hr	12 hr	24hr
1	0.38	0.82	1.53	1.87	2.06	2.32	2.82	3.30
2	0.46	0.98	1.78	2.24	2.39	2.75	3.53	4.11
5	0.54	1.16	2.29	2.83	3.17	3.71	4.03	5.22
10	0.61	1.30	2.67	3.24	3.58	4.38	5.23	6.08
25	0.70	1.50	3.09	3.73	4.14	5.08	6.08	7.10
50	0.78	1.66	3.48	4.20	4.62	5.62	6.78	7.91
100	0.85	1.82	3.86	4.68	5.19	6.21	7.45	8.79
500	1.10	2.35	4.99	6.05	6.71	8.03	9.64	11.37

These precipitation depths serve as input data into the hydrologic model, and were routed through the model as described previously. According to standard engineering practice, flood routings were started at the lowest spillway crest elevation for each dam. This corresponds to elevation 1144.0 ft-msl and 1142.5 ft-msl for the Primary and Secondary Ash Ponds, respectively. The results of the 10-year, 25-year, and 100-year storm events are shown in Table 6.

Table 6 – Frequency Model Results

	Peak Elevation (ft-msl)	Peak Inflow (cfs)	Peak Outflow (cfs)
10-Year Storm Results			
Primary	1148.55	1718.09	1700.04
Secondary	1147.33	1721.99	1706.83
25-Year Storm Results			
Primary	1148.94	2169.32	2149.13
Secondary	1147.75	2175.62	2156.64
100-Year Storm Results			
Primary	1149.48	2933.96	2862.69
Secondary	1148.35	2893.87	2874.07

2.6 PMF MODEL RESULTS

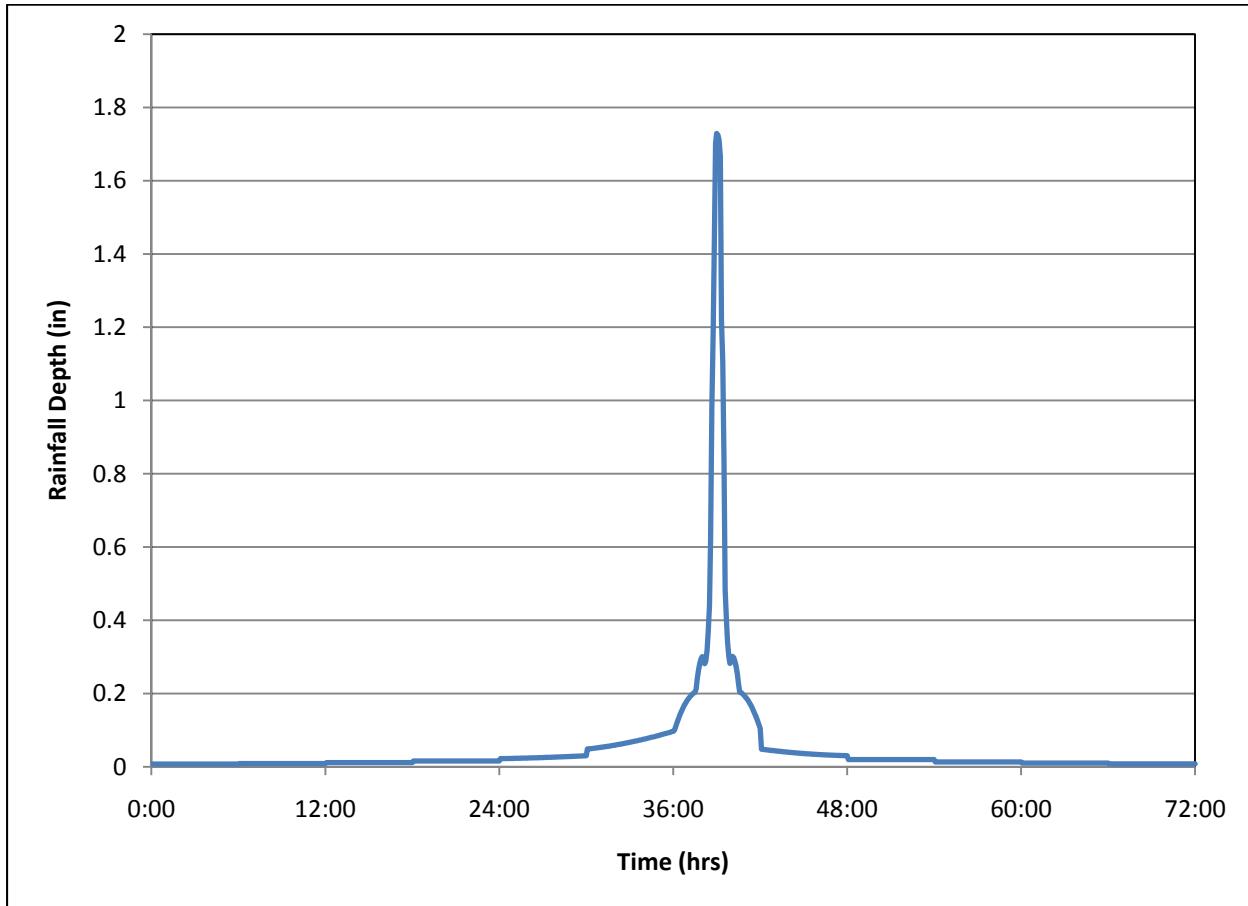
The Probable Maximum Flood (PMF) is defined as the greatest flood to be expected, and the Probable Maximum Precipitation (PMP) is theoretically the greatest depth of rainfall for a given duration that is physically possible over a given size storm area at a particular geographic location. Generally, the rainfall depth is calculated for the ten square miles of the watershed which receive the highest intensity rainfall.

Hydrometeorological Report No. 52 (HMR-52),⁷ developed by the U.S. Army Corps of Engineers, was used to determine the rainfall for each basin. PMP estimates were taken from Hydrometeorological Report No. 51⁸ and distributed according to HMR-52 to obtain average rainfall depths over the various drainage areas.

HMR-52 calculates rainfall depths for storm durations ranging from five minutes to seventy-two hours. Table 7 lists the point rainfall depths calculated by HMR-52 for storm durations from one hour to 72 hours. Because the total drainage area is less than ten square miles, the same rainfall depths were applied to both basins. HMR-52 also produces a 72-hour, critically stacked temporal distribution by arranging the incremental rainfall depths to produce the rainfall hyetograph shown in Figure 7.

Table 7 – HMR-52 Point Rainfall Depths

Storm Duration (hr)	Depth (in)
1	15.89
2	19.98
3	23.22
6	29.14
12	34.10
24	38.61
48	42.92
72	45.40

**Figure 7 – PMP Rainfall Hyetograph**

The PMF was modeled as described previously, with flood routing started at the lowest spillway crest elevation – 1144.0 ft-msl and 1142.5 ft-msl for the Primary and Secondary Ash Ponds, respectively. Additionally, the 25% and 50% PMF were calculated for the two Ponds. Table 8 contains the results of these PMF model runs – the 25% PMF, 50% PMF, and 100% PMF, respectively.

Table 8 –PMF Model Results

	Peak Elevation (ft-msl)	Peak Inflow (cfs)	Peak Outflow (cfs)
25% PMF Results			
Primary	1150.04	3757.22	3627.15
Secondary	1148.94	3670.68	3660.88
50% PMF Results			
Primary	1151.96	7501.95	6713.03
Secondary	1150.80	6787.60	6764.12
100% PMF Results			
Primary	1154.87	14991.39	12803.89
Secondary	1153.45	13008.71	12600.25

3.0 SUMMARY AND CONCLUSIONS

Based on the results of the hydraulic analysis, both dams are hydraulically adequate for the full range of storm events from the 10-year to the 100% PMF event. Table 9 lists the pertinent elevation data for each dam, including the top of dam elevation and principal and emergency spillway crest elevations. Comparing these elevations to the maximum water surface elevations shown in Table 10 indicates that each dam would safely contain all flood events up to, and including, the 100% PMF. Additionally, the emergency spillway for both dams is engaged somewhat frequently, even during a storm event as low as the 10-year storm. This should have no adverse affects on these structures, as they appear to be designed to withstand frequent engaging.

Table 9 – Pertinent Dam Information

	Top of Dam (ft-msl)	Principal Spillway (ft-msl)	Emergency Spillway (ft-msl)
Primary	1155.00	1144.00	1146.00
Secondary	1155.00	1142.50	1145.00

Table 10 – Summary of Results

	10-year	25-year	100-year	25% PMF	50% PMF	100% PMF
Primary	1148.55	1148.94	1149.48	1150.04	1151.96	1154.87
Secondary	1147.33	1147.75	1148.35	1148.94	1150.80	1153.45

It should be noted that these results reflect the best understanding of existing conditions and could be significantly affected by major changes to either of the reservoirs. The assumptions in this analysis represent average reservoir conditions. In their current conditions, the Primary Ash and Secondary Ash Ponds associated with the Flint Creek Power Plant are deemed to be hydraulically adequate for any storm event up to, and including, the 100% PMF. Pertinent drawings for existing conditions are included in Appendix C.

Appendix A References

References

1. U.S. Army Corps of Engineers, Hydrologic Engineering Center: *Hydrologic Modeling System HEC-HMS - User's Manual Version 3.4*, Davis, California, August 2009.
2. "Soil Data Mart." *NRCS Soil Survey Geographic (SSURGO) Database*.
<http://soildatamart.nrcs.usda.gov>.
3. "National Land Cover Dataset 2001." *USGS Seamless Data Warehouse*. August 30, 2010.
<http://seamless.usgs.gov/nlcd.php>.
4. U.S. Army Corps of Engineers, Hydrologic Engineering Center: *River Analysis System - User's Manual Version 4.1*, Davis, California, January 2010.
5. U.S. Department of Commerce, National Oceanic and Atmospheric Administration: *Technical Memorandum NWS HYDRO-35, Five- to 60-Minute Precipitation Frequency for the Eastern and Central United States*, Silver Spring, MD, June 1977.
6. U.S. Department of Commerce, Weather Bureau: *Technical Paper No. 40, Rainfall Frequency Atlas of the United States for Durations from 30 Minutes to 24 Hours and Return Periods from 1 to 100 Years*, Washington, D.C., May 1961.
7. U.S. Department of Commerce, National Oceanic and Atmospheric Administration and U.S. Department of the Army, Corps of Engineers: *Hydrometeorological Report No. 52, Application of Probable Maximum Precipitation Estimates, United States East of the 105th Meridian*, Washington, D.C., 1982.
8. U.S. Department of Commerce, National Oceanic and Atmospheric Administration and U.S. Department of the Army, Corps of Engineers: *Hydrometeorological Report No. 51, Probable Maximum Precipitation Estimates, United States East of the 105th Meridian*, Washington, D.C., 1978.

Appendix B
Discharge Rating Curve Calculations and Hydrologic Parameters

Primary Ash Pond

Secondary Ash Pond

Weir Box

Elevation [ft-msl]	Discharge [cfs]
1144	0.00
1145	12.65
1146	33.91

$$Q = 3.33(L-0.2H)H^{1.5}$$

L 4 ft

Weir Box (from Plans)

Elevation [ft-msl]	Discharge [MGD]	Discharge [cfs]
1142.50	0.000	0.000
1142.70	2.495	3.860
1142.88	6.387	9.882
1143.95	47.806	73.967
1145.00	106.332	164.520

Assumed rectangular, sharp-crested weir equation with end contractions accounted for; congruent with calculations made by AEP.

Values taken from plans for design of new spillway; linear interpolation between points.

HEC-RAS Plan: PrimarySpwy River: Primary Reach: Spwy

Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Spwy	3045	PF 1	1.00	1144.00	1146.06		1146.06	0.000000	0.00	2396.65	1180.88	0.00
Spwy	3045	PF 2	10.00	1144.00	1146.19		1146.19	0.000000	0.00	2545.85	1183.29	0.00
Spwy	3045	PF 3	50.00	1144.00	1146.41		1146.41	0.000000	0.02	2801.38	1187.40	0.00
Spwy	3045	PF 4	100.00	1144.00	1146.57		1146.57	0.000000	0.03	2995.43	1190.52	0.00
Spwy	3045	PF 5	250.00	1144.00	1146.91		1146.91	0.000002	0.07	3400.39	1196.99	0.01
Spwy	3045	PF 6	500.00	1144.00	1147.32		1147.32	0.000004	0.13	3889.13	1204.76	0.01
Spwy	3045	PF 7	750.00	1144.00	1147.64		1147.64	0.000006	0.18	4282.36	1210.97	0.02
Spwy	3045	PF 8	1000.00	1144.00	1147.93		1147.93	0.000009	0.22	4629.79	1216.44	0.02
Spwy	3045	PF 9	1500.00	1144.00	1148.42		1148.42	0.000013	0.29	5229.47	1225.81	0.02
Spwy	3045	PF 10	2000.00	1144.00	1148.85		1148.85	0.000017	0.35	5757.49	1234.00	0.03
Spwy	3045	PF 11	2500.00	1144.00	1149.20		1149.21	0.000021	0.41	6196.28	1240.77	0.03
Spwy	3045	PF 12	3000.00	1144.00	1149.57		1149.57	0.000024	0.46	6653.47	1247.78	0.03
Spwy	3045	PF 13	3500.00	1144.00	1149.93		1149.93	0.000027	0.50	7100.37	1254.60	0.04
Spwy	3045	PF 14	4000.00	1144.00	1150.27		1150.27	0.000029	0.54	7531.30	1259.46	0.04
Spwy	3045	PF 15	5000.00	1144.00	1150.91		1150.92	0.000032	0.61	8344.34	1267.72	0.04
Spwy	3045	PF 16	6000.00	1144.00	1151.51		1151.52	0.000035	0.68	9101.64	1275.37	0.04
Spwy	3045	PF 17	7000.00	1144.00	1152.07		1152.07	0.000038	0.73	9813.08	1282.51	0.05
Spwy	3045	PF 18	8000.00	1144.00	1152.59		1152.60	0.000040	0.79	10485.21	1289.22	0.05
Spwy	3045	PF 19	9000.00	1144.00	1153.08		1153.09	0.000042	0.83	11123.51	1295.51	0.05
Spwy	3045	PF 20	10000.00	1144.00	1153.55		1153.56	0.000043	0.88	11734.40	1301.50	0.05
Spwy	3045	PF 21	11000.00	1144.00	1154.00		1154.01	0.000045	0.92	12321.14	1307.22	0.05
Spwy	3045	PF 22	12000.00	1144.00	1154.43		1154.45	0.000046	0.96	12886.09	1312.70	0.05
Spwy	3045	PF 23	13000.00	1144.00	1154.85		1154.87	0.000047	1.00	13436.67	1318.01	0.05
Spwy	3045	PF 24	14000.00	1144.00	1155.25		1155.27	0.000048	1.04	13967.30	1321.54	0.05
Spwy	3045	PF 25	15000.00	1144.00	1155.64		1155.66	0.000050	1.07	14482.41	1324.08	0.06
Spwy	2645	PF 1	1.00	1144.00	1146.06		1146.06	0.000000	0.00	1052.44	540.92	0.00
Spwy	2645	PF 2	10.00	1144.00	1146.19		1146.19	0.000000	0.01	1120.95	544.71	0.00
Spwy	2645	PF 3	50.00	1144.00	1146.41		1146.41	0.000001	0.04	1239.01	551.19	0.00
Spwy	2645	PF 4	100.00	1144.00	1146.57		1146.57	0.000002	0.08	1329.23	556.09	0.01
Spwy	2645	PF 5	250.00	1144.00	1146.91		1146.91	0.000008	0.17	1518.91	566.24	0.02
Spwy	2645	PF 6	500.00	1144.00	1147.31		1147.31	0.000021	0.30	1750.30	578.39	0.03
Spwy	2645	PF 7	750.00	1144.00	1147.63		1147.64	0.000034	0.41	1938.54	588.09	0.04
Spwy	2645	PF 8	1000.00	1144.00	1147.92		1147.92	0.000046	0.50	2106.30	596.60	0.04
Spwy	2645	PF 9	1500.00	1144.00	1148.40		1148.41	0.000069	0.66	2399.40	611.18	0.06
Spwy	2645	PF 10	2000.00	1144.00	1148.83		1148.84	0.000089	0.80	2661.06	623.92	0.06
Spwy	2645	PF 11	2500.00	1144.00	1149.17		1149.19	0.000109	0.93	2880.32	634.39	0.07
Spwy	2645	PF 12	3000.00	1144.00	1149.54		1149.55	0.000124	1.04	3112.38	645.29	0.08
Spwy	2645	PF 13	3500.00	1144.00	1149.89		1149.91	0.000136	1.13	3341.89	655.89	0.08
Spwy	2645	PF 14	4000.00	1144.00	1150.23		1150.25	0.000146	1.22	3565.68	664.73	0.09
Spwy	2645	PF 15	5000.00	1144.00	1150.87		1150.89	0.000162	1.36	3993.34	679.27	0.09
Spwy	2645	PF 16	6000.00	1144.00	1151.45		1151.49	0.000174	1.49	4397.45	692.66	0.10
Spwy	2645	PF 17	7000.00	1144.00	1152.01		1152.04	0.000183	1.61	4782.14	705.17	0.10
Spwy	2645	PF 18	8000.00	1144.00	1152.52		1152.56	0.000192	1.72	5151.01	722.17	0.10
Spwy	2645	PF 19	9000.00	1144.00	1153.01		1153.06	0.000199	1.82	5507.16	734.88	0.11
Spwy	2645	PF 20	10000.00	1144.00	1153.48		1153.53	0.000205	1.91	5852.04	746.26	0.11
Spwy	2645	PF 21	11000.00	1144.00	1153.92		1153.98	0.000211	1.99	6186.67	758.37	0.11
Spwy	2645	PF 22	12000.00	1144.00	1154.35		1154.41	0.000216	2.07	6513.57	773.21	0.11
Spwy	2645	PF 23	13000.00	1144.00	1154.76		1154.83	0.000220	2.15	6836.81	787.62	0.12
Spwy	2645	PF 24	14000.00	1144.00	1155.16		1155.23	0.000224	2.22	7152.83	800.63	0.12
Spwy	2645	PF 25	15000.00	1144.00	1155.55		1155.62	0.000228	2.29	7463.77	812.08	0.12
Spwy	2090	PF 1	1.00	1144.00	1146.06		1146.06	0.000000	0.00	439.34	223.25	0.00
Spwy	2090	PF 2	10.00	1144.00	1146.19		1146.19	0.000000	0.02	467.60	224.52	0.00
Spwy	2090	PF 3	50.00	1144.00	1146.41		1146.41	0.000003	0.10	516.01	226.69	0.01
Spwy	2090	PF 4	100.00	1144.00	1146.57		1146.57	0.000011	0.19	552.70	228.31	0.02
Spwy	2090	PF 5	250.00	1144.00	1146.90		1146.90	0.000046	0.41	628.44	231.63	0.04
Spwy	2090	PF 6	500.00	1144.00	1147.28		1147.29	0.000121	0.72	718.48	235.51	0.07
Spwy	2090	PF 7	750.00	1144.00	1147.58		1147.60	0.000202	0.99	790.00	238.55	0.09
Spwy	2090	PF 8	1000.00	1144.00	1147.84		1147.87	0.000282	1.22	852.81	241.18	0.11
Spwy	2090	PF 9	1500.00	1144.00	1148.29		1148.33	0.000436	1.64	960.49	245.64	0.14
Spwy	2090	PF 10	2000.00	1144.00	1148.67		1148.73	0.000578	2.00	1055.05	249.48	0.16
Spwy	2090	PF 11	2500.00	1144.00	1148.97		1149.05	0.000726	2.33	1131.50	252.55	0.18
Spwy	2090	PF 12	3000.00	1144.00	1149.30		1149.40	0.000840	2.62	1214.22	255.83	0.20
Spwy	2090	PF 13	3500.00	1144.00	1149.62		1149.74	0.000933	2.87	1296.46	259.04	0.21
Spwy	2090	PF 14	4000.00	1144.00	1149.93		1150.07	0.001013	3.10	1376.78	262.15	0.22
Spwy	2090	PF 15	5000.00	1144.00	1150.51		1150.69	0.001141	3.50	1530.63	266.97	0.24
Spwy	2090	PF 16	6000.00	1144.00	1151.05		1151.26	0.001243	3.85	1675.69	271.22	0.26
Spwy	2090	PF 17	7000.00	1144.00	1151.55		1151.80	0.001327	4.17	1813.49	275.19	0.27
Spwy	2090	PF 18	8000.00	1144.00	1152.02		1152.31	0.001400	4.46	1944.51	278.92	0.28
Spwy	2090	PF 19	9000.00	1144.00	1152.47		1152.79	0.001465	4.73	2069.87	282.44	0.29
Spwy	2090	PF 20	10000.00	1144.00	1152.90		1153.25	0.001521	4.98	2190.84	285.79	0.29
Spwy	2090	PF 21	11000.00	1144.00	1153.30		1153.69	0.001572	5.21	2307.63	288.99	0.30
Spwy	2090	PF 22	12000.00	1144.00	1153.69		1154.11	0.001618	5.43	2420.69	292.06	0.31
Spwy	2090	PF 23	13000.00	1144.00	1154.07		1154.53	0.001659	5.64	2531.70	295.04	0.31
Spwy	2090	PF 24	14000.00	1144.00	1154.43		1154.92	0.001697	5.85	2639.15	297.89	0.32
Spwy	2090	PF 25	15000.00	1144.00	1154.78		1155.30	0.001732	6.04	2744.33	300.66	0.32
Spwy	2040	PF 1	1.00	1146.00	1146.06		1146.06	0.000210	0.07	14.54	227.98	0.05

HEC-RAS Plan: PrimarySpwy River: Primary Reach: Spwy (Continued)

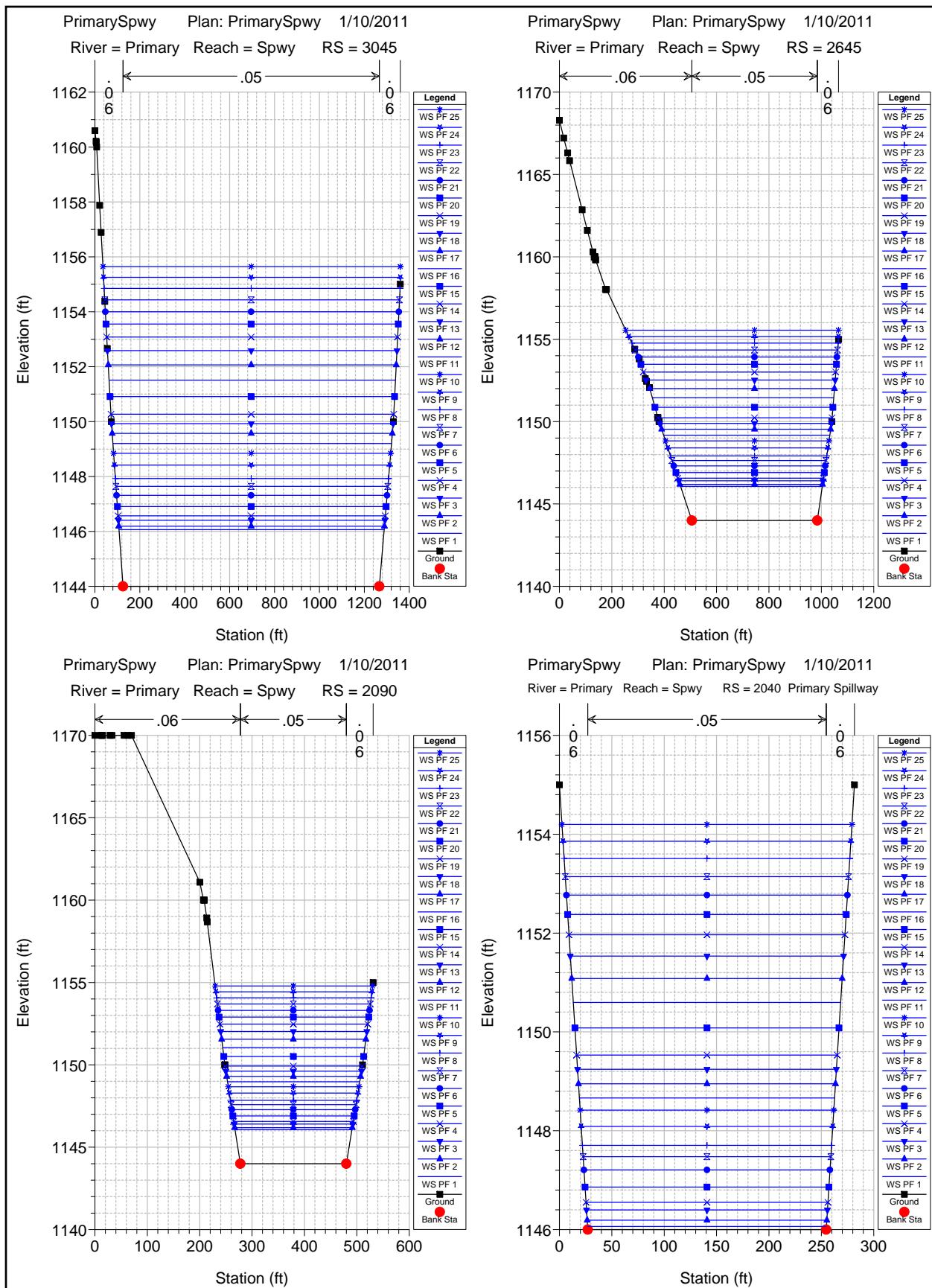
Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Spwy	2040	PF 2	10.00	1146.00	1146.19	1146.03	1146.19	0.000561	0.23	43.17	228.74	0.09
Spwy	2040	PF 3	50.00	1146.00	1146.40	1146.11	1146.40	0.001164	0.55	91.24	229.99	0.15
Spwy	2040	PF 4	100.00	1146.00	1146.55	1146.18	1146.56	0.001564	0.79	126.75	230.92	0.19
Spwy	2040	PF 5	250.00	1146.00	1146.86	1146.34	1146.88	0.002238	1.27	197.81	232.76	0.24
Spwy	2040	PF 6	500.00	1146.00	1147.21	1146.53	1147.26	0.002869	1.80	279.18	234.84	0.29
Spwy	2040	PF 7	750.00	1146.00	1147.47	1146.69	1147.55	0.003307	2.21	342.03	236.44	0.32
Spwy	2040	PF 8	1000.00	1146.00	1147.70	1146.84	1147.80	0.003616	2.55	396.54	237.82	0.34
Spwy	2040	PF 9	1500.00	1146.00	1148.09	1147.10	1148.24	0.004114	3.11	488.22	240.13	0.38
Spwy	2040	PF 10	2000.00	1146.00	1148.42	1147.33	1148.61	0.004462	3.58	567.90	242.11	0.41
Spwy	2040	PF 11	2500.00	1146.00	1148.66		1148.92	0.005035	4.05	627.48	243.58	0.44
Spwy	2040	PF 12	3000.00	1146.00	1148.95		1149.25	0.005131	4.38	697.84	245.31	0.45
Spwy	2040	PF 13	3500.00	1146.00	1149.24		1149.57	0.005078	4.64	769.82	247.06	0.45
Spwy	2040	PF 14	4000.00	1146.00	1149.53		1149.89	0.004979	4.86	841.06	248.79	0.46
Spwy	2040	PF 15	5000.00	1146.00	1150.08		1150.50	0.004769	5.24	978.69	252.08	0.46
Spwy	2040	PF 16	6000.00	1146.00	1150.60		1151.07	0.004591	5.57	1109.28	255.17	0.46
Spwy	2040	PF 17	7000.00	1146.00	1151.08		1151.60	0.004447	5.86	1233.52	258.08	0.46
Spwy	2040	PF 18	8000.00	1146.00	1151.53		1152.10	0.004339	6.12	1351.53	260.81	0.46
Spwy	2040	PF 19	9000.00	1146.00	1151.96		1152.58	0.004255	6.38	1464.21	263.39	0.46
Spwy	2040	PF 20	10000.00	1146.00	1152.38		1153.03	0.004186	6.61	1572.90	265.85	0.46
Spwy	2040	PF 21	11000.00	1146.00	1152.77		1153.47	0.004131	6.83	1677.54	268.20	0.46
Spwy	2040	PF 22	12000.00	1146.00	1153.14		1153.89	0.004086	7.05	1778.63	270.45	0.46
Spwy	2040	PF 23	13000.00	1146.00	1153.51		1154.29	0.004041	7.24	1878.01	272.65	0.47
Spwy	2040	PF 24	14000.00	1146.00	1153.86		1154.69	0.004009	7.44	1973.80	274.75	0.47
Spwy	2040	PF 25	15000.00	1146.00	1154.20		1155.07	0.003979	7.62	2067.42	276.79	0.47
Spwy	1980	PF 1	1.00	1146.00	1146.03	1146.03	1146.03	0.003444	0.16	6.28	227.77	0.17
Spwy	1980	PF 2	10.00	1146.00	1146.03	1146.03	1146.07	0.344390	1.59	6.28	227.77	1.69
Spwy	1980	PF 3	50.00	1146.00	1146.11	1146.11	1146.17	0.084653	1.99	25.15	228.26	1.06
Spwy	1980	PF 4	100.00	1146.00	1146.18	1146.18	1146.27	0.064245	2.42	41.44	228.69	1.00
Spwy	1980	PF 5	250.00	1146.00	1146.34	1146.34	1146.50	0.052170	3.27	76.55	229.61	1.00
Spwy	1980	PF 6	500.00	1146.00	1146.53	1146.53	1146.79	0.045516	4.14	121.08	230.77	1.00
Spwy	1980	PF 7	750.00	1146.00	1146.69	1146.69	1147.04	0.041388	4.73	159.14	231.76	1.00
Spwy	1980	PF 8	1000.00	1146.00	1146.84	1146.84	1147.26	0.039052	5.22	192.70	232.62	1.00
Spwy	1980	PF 9	1500.00	1146.00	1147.10	1147.10	1147.65	0.035580	5.96	253.33	234.18	1.00
Spwy	1980	PF 10	2000.00	1146.00	1147.33	1147.33	1147.99	0.033347	6.56	307.60	235.57	1.00
Spwy	1980	PF 11	2500.00	1146.00	1147.83		1148.37	0.017832	5.93	426.22	238.57	0.77
Spwy	1980	PF 12	3000.00	1146.00	1148.25		1148.76	0.012783	5.77	527.41	241.10	0.68
Spwy	1980	PF 13	3500.00	1146.00	1148.63		1149.13	0.010361	5.76	618.18	243.35	0.63
Spwy	1980	PF 14	4000.00	1146.00	1148.97		1149.48	0.008942	5.80	702.13	245.41	0.59
Spwy	1980	PF 15	5000.00	1146.00	1149.59		1150.13	0.007361	5.98	855.59	249.14	0.56
Spwy	1980	PF 16	6000.00	1146.00	1150.15		1150.73	0.006504	6.19	995.35	252.48	0.54
Spwy	1980	PF 17	7000.00	1146.00	1150.66		1151.28	0.005964	6.40	1125.52	255.55	0.52
Spwy	1980	PF 18	8000.00	1146.00	1151.13		1151.80	0.005605	6.62	1247.33	258.40	0.51
Spwy	1980	PF 19	9000.00	1146.00	1151.58		1152.28	0.005352	6.84	1362.45	261.06	0.51
Spwy	1980	PF 20	10000.00	1146.00	1152.00		1152.75	0.005155	7.04	1472.96	263.59	0.51
Spwy	1980	PF 21	11000.00	1146.00	1152.40		1153.19	0.005005	7.25	1578.75	265.98	0.50
Spwy	1980	PF 22	12000.00	1146.00	1152.78		1153.61	0.004888	7.44	1680.58	268.27	0.50
Spwy	1980	PF 23	13000.00	1146.00	1153.15		1154.02	0.004778	7.62	1780.75	270.50	0.50
Spwy	1980	PF 24	14000.00	1146.00	1153.50		1154.42	0.004696	7.81	1876.85	272.62	0.50
Spwy	1980	PF 25	15000.00	1146.00	1153.85		1154.80	0.004624	7.98	1970.72	274.68	0.50
Spwy	1865	PF 1	1.00	1142.50	1145.03		1145.03	0.000000	0.00	590.46	242.39	0.00
Spwy	1865	PF 2	10.00	1142.50	1145.06		1145.06	0.000000	0.02	598.04	242.61	0.00
Spwy	1865	PF 3	50.00	1142.50	1145.17		1145.17	0.000002	0.08	625.80	243.39	0.01
Spwy	1865	PF 4	100.00	1142.50	1145.28		1145.28	0.000007	0.16	652.10	244.13	0.02
Spwy	1865	PF 5	250.00	1142.50	1145.55		1145.56	0.000032	0.36	719.08	246.00	0.04
Spwy	1865	PF 6	500.00	1142.50	1145.94		1145.95	0.000086	0.63	815.36	248.66	0.06
Spwy	1865	PF 7	750.00	1142.50	1146.28		1146.30	0.000140	0.86	900.88	251.00	0.08
Spwy	1865	PF 8	1000.00	1142.50	1146.59		1146.61	0.000192	1.05	978.04	253.10	0.09
Spwy	1865	PF 9	1500.00	1142.50	1147.13		1147.16	0.000283	1.39	1115.26	256.78	0.11
Spwy	1865	PF 10	2000.00	1142.50	1147.60		1147.64	0.000363	1.68	1236.37	259.99	0.13
Spwy	1865	PF 11	2500.00	1142.50	1148.02		1148.07	0.000433	1.93	1345.86	262.85	0.14
Spwy	1865	PF 12	3000.00	1142.50	1148.40		1148.47	0.000495	2.16	1447.21	265.48	0.16
Spwy	1865	PF 13	3500.00	1142.50	1148.75		1148.84	0.000553	2.37	1541.43	267.90	0.17
Spwy	1865	PF 14	4000.00	1142.50	1149.09		1149.18	0.000605	2.57	1630.75	270.17	0.18
Spwy	1865	PF 15	5000.00	1142.50	1149.69		1149.82	0.000697	2.92	1796.52	274.33	0.19
Spwy	1865	PF 16	6000.00	1142.50	1150.25		1150.40	0.000778	3.25	1949.23	278.15	0.21
Spwy	1865	PF 17	7000.00	1142.50	1150.76		1150.94	0.000850	3.54	2092.02	281.40	0.22
Spwy	1865	PF 18	8000.00	1142.50	1151.23		1151.45	0.000914	3.81	2226.56	284.42	0.23
Spwy	1865	PF 19	9000.00	1142.50	1151.68		1151.92	0.000974	4.07	2354.06	287.25	0.24
Spwy	1865	PF 20	10000.00	1142.50	1152.10		1152.38	0.001027	4.30	2476.62	289.95	0.24
Spwy	1865	PF 21	11000.00	1142.50	1152.51		1152.81	0.001077	4.53	2594.15	292.51	0.25
Spwy	1865	PF 22	12000.00	1142.50	1152.89		1153.22	0.001124	4.75	2707.35	294.96	0.26
Spwy	1865	PF 23	13000.00	1142.50	1153.27		1153.63	0.001166	4.95	2818.73	297.36	0.27
Spwy	1865	PF 24	14000.00	1142.50	1153.63		1154.01	0.001206	5.14	2925.72	299.65	0.27
Spwy	1865	PF 25	15000.00	1142.50	1153.97		1154.39	0.001244	5.33	3030.20	301.87	0.28
Spwy	1575	PF 1	1.00	1142.50	1145.03		1145.03	0.000000	0.00	924.26	390.65	0.00
Spwy	1575	PF 2	10.00	1142.50	1145.06		1145.06	0.000000	0.01	936.47	391.27	0.00

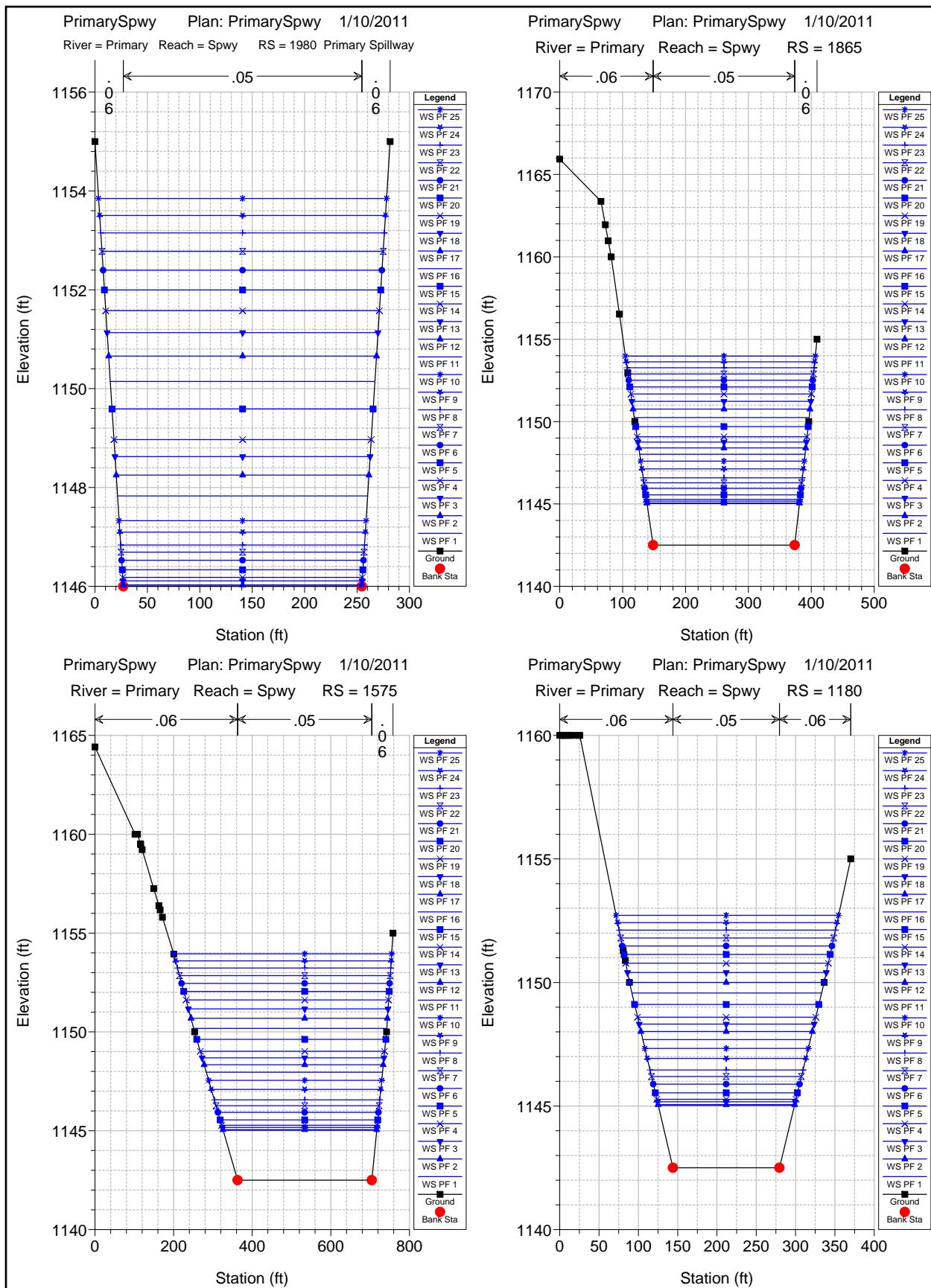
HEC-RAS Plan: PrimarySpwy River: Primary Reach: Spwy (Continued)

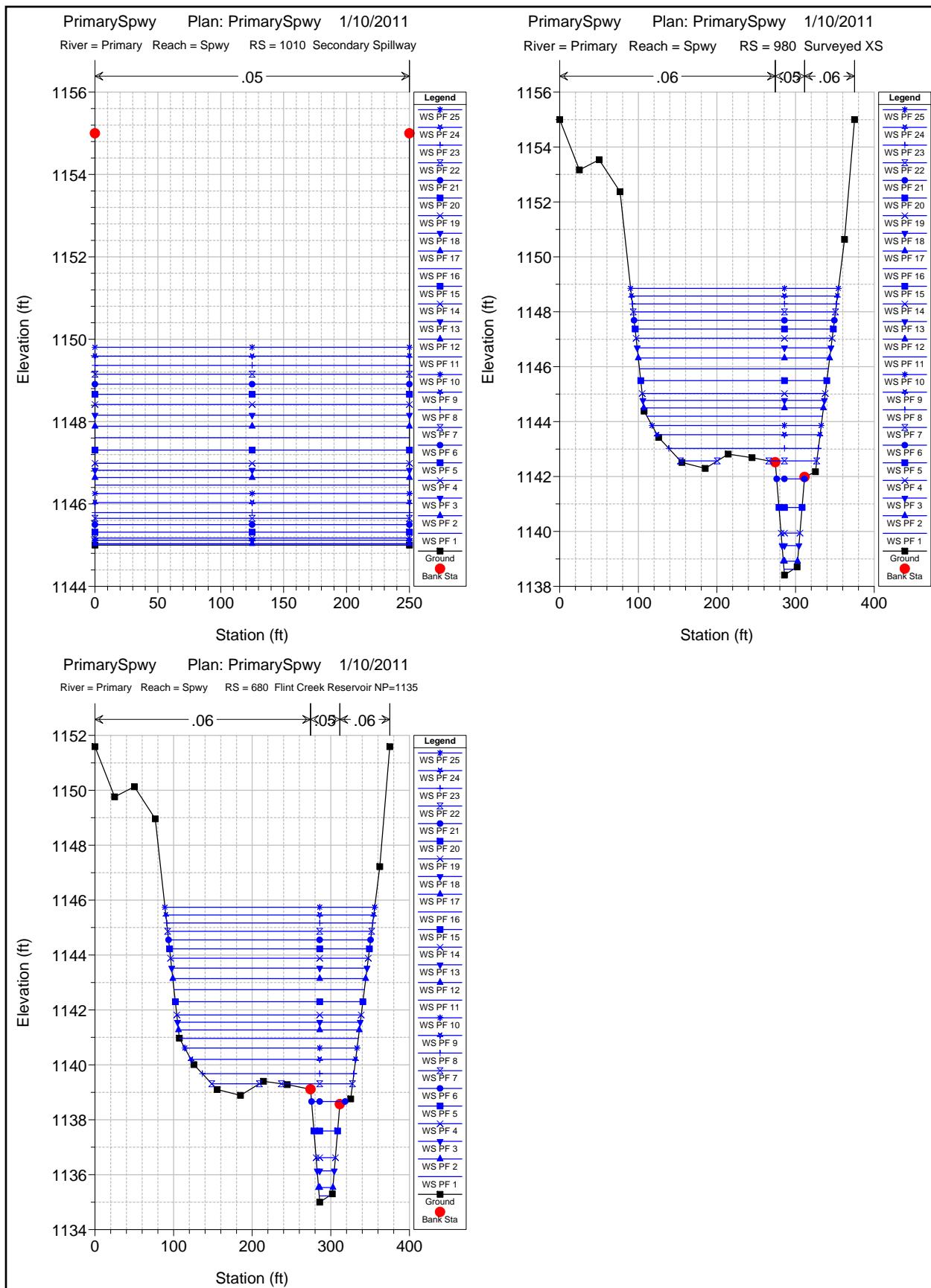
Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Spwy	1575	PF 3	50.00	1142.50	1145.17		1145.17	0.000001	0.05	981.21	393.50	0.01
Spwy	1575	PF 4	100.00	1142.50	1145.28		1145.28	0.000003	0.10	1023.45	395.60	0.01
Spwy	1575	PF 5	250.00	1142.50	1145.55		1145.55	0.000014	0.23	1130.74	400.88	0.02
Spwy	1575	PF 6	500.00	1142.50	1145.93		1145.93	0.000036	0.41	1285.04	408.36	0.04
Spwy	1575	PF 7	750.00	1142.50	1146.26		1146.27	0.000059	0.55	1422.82	414.92	0.05
Spwy	1575	PF 8	1000.00	1142.50	1146.56		1146.57	0.000081	0.68	1547.84	420.78	0.06
Spwy	1575	PF 9	1500.00	1142.50	1147.09		1147.10	0.000119	0.90	1772.31	431.11	0.07
Spwy	1575	PF 10	2000.00	1142.50	1147.55		1147.57	0.000152	1.08	1972.73	440.13	0.08
Spwy	1575	PF 11	2500.00	1142.50	1147.96		1147.99	0.000181	1.24	2155.83	448.21	0.09
Spwy	1575	PF 12	3000.00	1142.50	1148.34		1148.37	0.000206	1.38	2326.95	455.63	0.10
Spwy	1575	PF 13	3500.00	1142.50	1148.69		1148.72	0.000229	1.52	2487.39	462.48	0.11
Spwy	1575	PF 14	4000.00	1142.50	1149.02		1149.06	0.000249	1.64	2640.71	468.94	0.11
Spwy	1575	PF 15	5000.00	1142.50	1149.63		1149.68	0.000285	1.86	2928.41	480.81	0.12
Spwy	1575	PF 16	6000.00	1142.50	1150.18		1150.24	0.000315	2.05	3196.82	491.14	0.13
Spwy	1575	PF 17	7000.00	1142.50	1150.69		1150.76	0.000341	2.23	3450.35	499.74	0.14
Spwy	1575	PF 18	8000.00	1142.50	1151.17		1151.25	0.000364	2.39	3691.22	507.78	0.14
Spwy	1575	PF 19	9000.00	1142.50	1151.62		1151.71	0.000384	2.54	3921.40	515.34	0.15
Spwy	1575	PF 20	10000.00	1142.50	1152.05		1152.15	0.000402	2.68	4144.25	522.56	0.15
Spwy	1575	PF 21	11000.00	1142.50	1152.46		1152.57	0.000419	2.81	4359.42	529.44	0.16
Spwy	1575	PF 22	12000.00	1142.50	1152.85		1152.97	0.000434	2.94	4568.04	536.02	0.16
Spwy	1575	PF 23	13000.00	1142.50	1153.23		1153.36	0.000447	3.06	4774.53	542.46	0.16
Spwy	1575	PF 24	14000.00	1142.50	1153.60		1153.74	0.000459	3.17	4974.04	548.60	0.17
Spwy	1575	PF 25	15000.00	1142.50	1153.95		1154.10	0.000470	3.27	5169.91	554.60	0.17
Spwy	1180	PF 1	1.00	1142.50	1145.03		1145.03	0.000000	0.00	390.05	173.31	0.00
Spwy	1180	PF 2	10.00	1142.50	1145.06		1145.06	0.000000	0.03	395.47	173.78	0.00
Spwy	1180	PF 3	50.00	1142.50	1145.17		1145.17	0.000005	0.13	415.21	175.47	0.01
Spwy	1180	PF 4	100.00	1142.50	1145.28		1145.28	0.000018	0.25	433.70	177.04	0.03
Spwy	1180	PF 5	250.00	1142.50	1145.53		1145.54	0.000081	0.56	479.83	180.89	0.06
Spwy	1180	PF 6	500.00	1142.50	1145.89		1145.90	0.000219	0.99	544.83	186.19	0.10
Spwy	1180	PF 7	750.00	1142.50	1146.19		1146.22	0.000364	1.35	602.33	190.76	0.12
Spwy	1180	PF 8	1000.00	1142.50	1146.46		1146.50	0.000505	1.67	654.19	194.78	0.15
Spwy	1180	PF 9	1500.00	1142.50	1146.93		1147.00	0.000765	2.22	746.99	201.78	0.19
Spwy	1180	PF 10	2000.00	1142.50	1147.33		1147.43	0.000996	2.68	829.61	207.82	0.21
Spwy	1180	PF 11	2500.00	1142.50	1147.69		1147.83	0.001204	3.09	904.92	213.17	0.24
Spwy	1180	PF 12	3000.00	1142.50	1148.02		1148.19	0.001392	3.46	975.38	218.06	0.26
Spwy	1180	PF 13	3500.00	1142.50	1148.32		1148.52	0.001566	3.80	1041.24	222.53	0.28
Spwy	1180	PF 14	4000.00	1142.50	1148.60		1148.83	0.001724	4.12	1104.34	226.73	0.29
Spwy	1180	PF 15	5000.00	1142.50	1149.11		1149.41	0.002006	4.69	1222.69	234.41	0.32
Spwy	1180	PF 16	6000.00	1142.50	1149.58		1149.94	0.002253	5.20	1333.24	241.36	0.34
Spwy	1180	PF 17	7000.00	1142.50	1150.00		1150.44	0.002469	5.66	1438.18	247.77	0.36
Spwy	1180	PF 18	8000.00	1142.50	1150.40		1150.90	0.002657	6.08	1538.34	252.83	0.38
Spwy	1180	PF 19	9000.00	1142.50	1150.78		1151.34	0.002828	6.47	1634.01	257.57	0.40
Spwy	1180	PF 20	10000.00	1142.50	1151.14		1151.76	0.002981	6.83	1726.93	262.23	0.41
Spwy	1180	PF 21	11000.00	1142.50	1151.48		1152.16	0.003123	7.17	1816.65	266.69	0.42
Spwy	1180	PF 22	12000.00	1142.50	1151.80		1152.54	0.003253	7.50	1903.74	270.94	0.43
Spwy	1180	PF 23	13000.00	1142.50	1152.12		1152.92	0.003362	7.79	1991.10	275.14	0.44
Spwy	1180	PF 24	14000.00	1142.50	1152.42		1153.28	0.003469	8.08	2075.19	279.12	0.45
Spwy	1180	PF 25	15000.00	1142.50	1152.72		1153.62	0.003565	8.36	2158.05	282.98	0.46
Spwy	1010	PF 1	1.00	1145.00	1145.03	1145.03	1145.03	0.003648	0.16	6.41	250.00	0.17
Spwy	1010	PF 2	10.00	1145.00	1145.04	1145.04	1145.06	0.120220	1.12	8.94	250.00	1.04
Spwy	1010	PF 3	50.00	1145.00	1145.12	1145.12	1145.16	0.054496	1.68	29.79	250.00	0.86
Spwy	1010	PF 4	100.00	1145.00	1145.17	1145.17	1145.26	0.065680	2.34	42.69	250.00	1.00
Spwy	1010	PF 5	250.00	1145.00	1145.32	1145.32	1145.47	0.053522	3.18	78.70	250.00	1.00
Spwy	1010	PF 6	500.00	1145.00	1145.50	1145.50	1145.75	0.046081	4.00	124.85	250.00	1.00
Spwy	1010	PF 7	750.00	1145.00	1145.65	1145.65	1145.98	0.042967	4.61	162.69	250.00	1.01
Spwy	1010	PF 8	1000.00	1145.00	1145.79	1145.79	1146.19	0.040015	5.06	197.60	250.00	1.00
Spwy	1010	PF 9	1500.00	1145.00	1146.04	1146.04	1146.56	0.036801	5.80	258.64	250.00	1.00
Spwy	1010	PF 10	2000.00	1145.00	1146.25	1146.25	1146.89	0.034722	6.39	312.99	250.00	1.01
Spwy	1010	PF 11	2500.00	1145.00	1146.46	1146.46	1147.19	0.032773	6.86	364.32	250.00	1.00
Spwy	1010	PF 12	3000.00	1145.00	1146.64	1146.64	1147.47	0.031783	7.31	410.43	250.00	1.01
Spwy	1010	PF 13	3500.00	1145.00	1146.82	1146.82	1147.74	0.030795	7.70	454.74	250.00	1.01
Spwy	1010	PF 14	4000.00	1145.00	1146.99	1146.99	1147.99	0.029938	8.05	497.13	250.00	1.01
Spwy	1010	PF 15	5000.00	1145.00	1147.31	1147.31	1148.47	0.028520	8.66	577.27	250.00	1.00
Spwy	1010	PF 16	6000.00	1145.00	1147.61	1147.61	1148.92	0.027434	9.20	652.16	250.00	1.00
Spwy	1010	PF 17	7000.00	1145.00	1147.89	1147.89	1149.35	0.026677	9.70	722.02	250.00	1.01
Spwy	1010	PF 18	8000.00	1145.00	1148.16	1148.16	1149.75	0.025989	10.14	789.06	250.00	1.01
Spwy	1010	PF 19	9000.00	1145.00	1148.42	1148.42	1150.14	0.025329	10.54	854.10	250.00	1.00
Spwy	1010	PF 20	10000.00	1145.00	1148.66	1148.66	1150.52	0.024854	10.92	915.71	250.00	1.01
Spwy	1010	PF 21	11000.00	1145.00	1148.91	1148.91	1150.88	0.024290	11.26	977.05	250.00	1.00
Spwy	1010	PF 22	12000.00	1145.00	1149.15	1149.15	1151.23	0.023703	11.56	1037.78	250.00	1.00
Spwy	1010	PF 23	13000.00	1145.00	1149.37	1149.37	1151.57	0.023587	11.91	1091.16	250.00	1.00
Spwy	1010	PF 24	14000.00	1145.00	1149.59	1149.59	1151.90	0.023226	12.21	1146.85	250.00	1.00
Spwy	1010	PF 25	15000.00	1145.00	1149.81	1149.81	1152.23	0.022894	12.49	1201.29	250.00	1.00
Spwy	980	PF 1	1.00	1138.41	1138.63		1138.64	0.012975	0.76	1.31	12.25	0.41
Spwy	980	PF 2	10.00	1138.41	1138.92		1138.96	0.012867	1.63	6.13	18.08	0.49
Spwy	980	PF 3	50.00	1138.41	1139.48		1139.61	0.012670	2.88	17.34	21.36	0.56

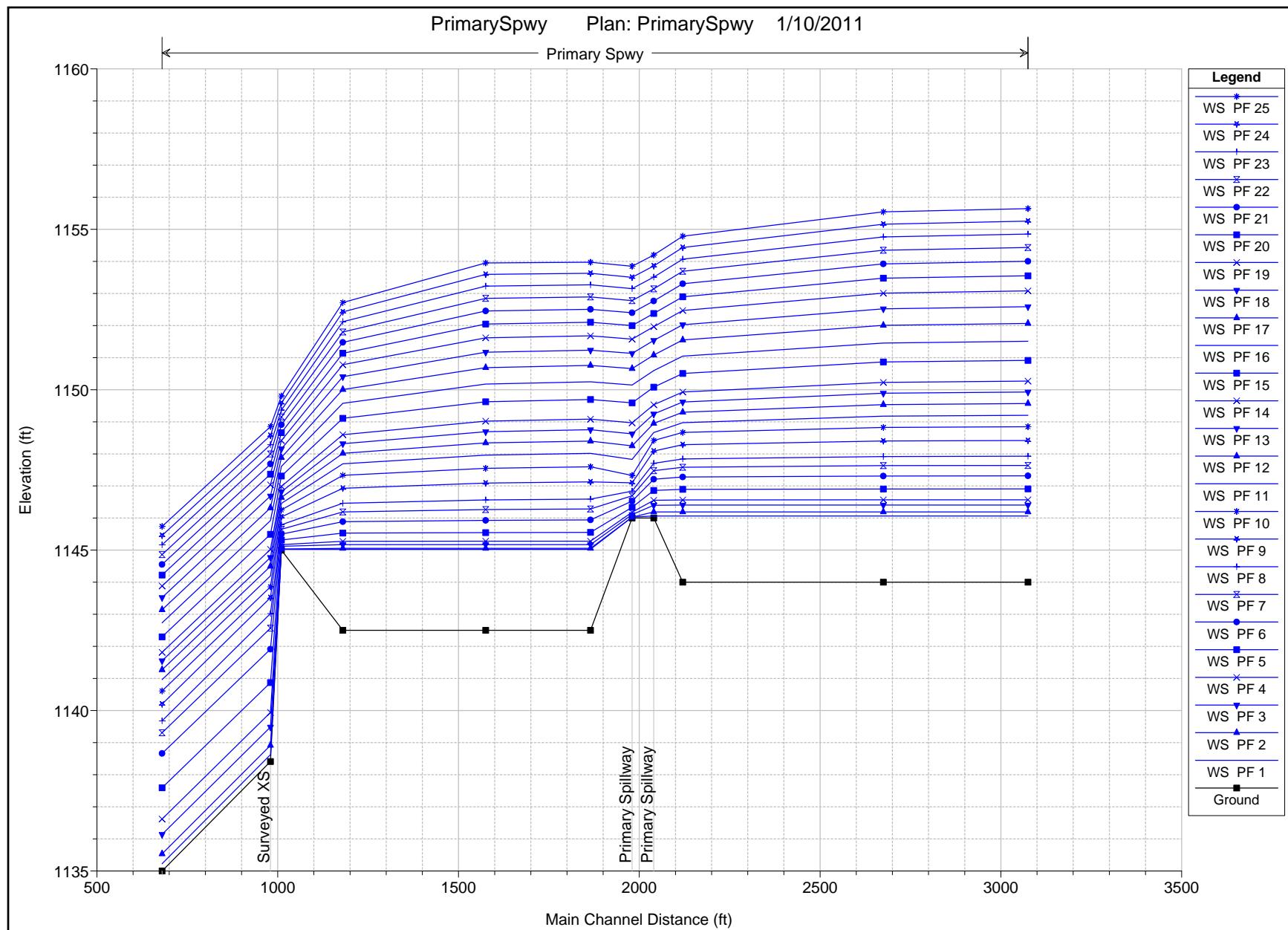
HEC-RAS Plan: Primary Spwy River: Primary Reach: Spwy (Continued)

Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Spwy	980	PF 4	100.00	1138.41	1139.94		1140.14	0.012492	3.61	27.71	23.99	0.59
Spwy	980	PF 5	250.00	1138.41	1140.87		1141.22	0.012257	4.76	52.49	29.35	0.63
Spwy	980	PF 6	500.00	1138.41	1141.91		1142.44	0.012095	5.80	86.24	35.37	0.65
Spwy	980	PF 7	750.00	1138.41	1142.57	1141.87	1143.24	0.012393	6.66	124.28	107.75	0.68
Spwy	980	PF 8	1000.00	1138.41	1143.02	1143.02	1143.69	0.011107	6.94	198.91	190.22	0.66
Spwy	980	PF 9	1500.00	1138.41	1143.52	1143.52	1144.24	0.011428	7.71	297.85	207.21	0.69
Spwy	980	PF 10	2000.00	1138.41	1143.86	1143.86	1144.66	0.012474	8.51	368.85	215.15	0.73
Spwy	980	PF 11	2500.00	1138.41	1144.20	1144.15	1145.03	0.012449	8.95	443.19	223.15	0.74
Spwy	980	PF 12	3000.00	1138.41	1144.50	1144.42	1145.37	0.012396	9.32	511.36	228.47	0.74
Spwy	980	PF 13	3500.00	1138.41	1144.77		1145.67	0.012316	9.63	574.02	230.71	0.75
Spwy	980	PF 14	4000.00	1138.41	1145.03		1145.96	0.012243	9.91	633.07	232.80	0.75
Spwy	980	PF 15	5000.00	1138.41	1145.49		1146.49	0.012123	10.43	742.92	236.65	0.76
Spwy	980	PF 16	6000.00	1138.41	1145.92		1146.99	0.012022	10.88	844.72	240.16	0.76
Spwy	980	PF 17	7000.00	1138.41	1146.32		1147.45	0.011945	11.29	940.23	243.40	0.77
Spwy	980	PF 18	8000.00	1138.41	1146.69		1147.89	0.011881	11.67	1030.88	246.44	0.77
Spwy	980	PF 19	9000.00	1138.41	1147.04		1148.31	0.011824	12.03	1117.69	249.32	0.78
Spwy	980	PF 20	10000.00	1138.41	1147.37		1148.72	0.011772	12.36	1201.29	252.06	0.78
Spwy	980	PF 21	11000.00	1138.41	1147.69		1149.10	0.011728	12.67	1282.05	254.67	0.79
Spwy	980	PF 22	12000.00	1138.41	1148.00		1149.47	0.011668	12.96	1361.15	257.21	0.79
Spwy	980	PF 23	13000.00	1138.41	1148.29		1149.83	0.011654	13.25	1436.48	259.61	0.79
Spwy	980	PF 24	14000.00	1138.41	1148.57		1150.18	0.011617	13.52	1510.97	261.95	0.80
Spwy	980	PF 25	15000.00	1138.41	1148.85		1150.52	0.011587	13.78	1583.58	264.22	0.80
Spwy	680	PF 1	1.00	1135.00	1135.23	1135.15	1135.23	0.009998	0.69	1.44	12.74	0.36
Spwy	680	PF 2	10.00	1135.00	1135.53	1135.37	1135.57	0.010009	1.51	6.63	18.24	0.44
Spwy	680	PF 3	50.00	1135.00	1136.14	1135.79	1136.25	0.010003	2.67	18.75	21.74	0.51
Spwy	680	PF 4	100.00	1135.00	1136.62	1136.14	1136.80	0.010019	3.35	29.87	24.51	0.53
Spwy	680	PF 5	250.00	1135.00	1137.59	1136.90	1137.90	0.010000	4.43	56.38	30.11	0.57
Spwy	680	PF 6	500.00	1135.00	1138.66	1137.78	1139.12	0.010009	5.44	92.27	42.73	0.60
Spwy	680	PF 7	750.00	1135.00	1139.31	1138.46	1139.88	0.010016	6.19	143.86	151.04	0.62
Spwy	680	PF 8	1000.00	1135.00	1139.68	1139.62	1140.28	0.010010	6.67	211.16	192.63	0.63
Spwy	680	PF 9	1500.00	1135.00	1140.20	1140.11	1140.83	0.010004	7.32	316.31	209.34	0.65
Spwy	680	PF 10	2000.00	1135.00	1140.61	1140.47	1141.27	0.010002	7.81	404.24	219.03	0.66
Spwy	680	PF 11	2500.00	1135.00	1140.96	1140.74	1141.65	0.010002	8.23	483.28	227.39	0.66
Spwy	680	PF 12	3000.00	1135.00	1141.27	1141.01	1141.99	0.010009	8.58	553.34	229.99	0.67
Spwy	680	PF 13	3500.00	1135.00	1141.55	1141.22	1142.31	0.010005	8.89	618.55	232.31	0.68
Spwy	680	PF 14	4000.00	1135.00	1141.82	1141.44	1142.60	0.010003	9.18	679.91	234.47	0.68
Spwy	680	PF 15	5000.00	1135.00	1142.30	1141.79	1143.15	0.010001	9.70	793.98	238.43	0.69
Spwy	680	PF 16	6000.00	1135.00	1142.74	1142.12	1143.66	0.010013	10.16	899.08	242.03	0.70
Spwy	680	PF 17	7000.00	1135.00	1143.14	1142.42	1144.14	0.010007	10.58	998.26	245.37	0.71
Spwy	680	PF 18	8000.00	1135.00	1143.52	1142.72	1144.59	0.010003	10.96	1092.37	248.50	0.71
Spwy	680	PF 19	9000.00	1135.00	1143.88	1142.98	1145.01	0.010003	11.32	1182.21	251.45	0.72
Spwy	680	PF 20	10000.00	1135.00	1144.23	1143.27	1145.42	0.010002	11.65	1268.66	254.26	0.72
Spwy	680	PF 21	11000.00	1135.00	1144.55	1143.55	1145.81	0.010001	11.96	1352.19	256.95	0.73
Spwy	680	PF 22	12000.00	1135.00	1144.86	1143.77	1146.19	0.010016	12.27	1432.41	259.50	0.73
Spwy	680	PF 23	13000.00	1135.00	1145.17	1144.01	1146.55	0.010012	12.55	1511.28	261.98	0.74
Spwy	680	PF 24	14000.00	1135.00	1145.46	1144.24	1146.90	0.010008	12.82	1588.16	264.38	0.74
Spwy	680	PF 25	15000.00	1135.00	1145.74	1144.47	1147.24	0.010005	13.08	1663.20	266.71	0.75









BASIN LAG TIME CALCULATION					
USING NRCS TR55 METHOD TO COMPUTE TIME OF CONCENTRATION					
Existing Conditions					
Project Data:	Comments:				
PROJECT	AEP10412				
LOCATION	Welsh Power Plant				
DATE	Dec-10				
BASIN COND.					
BY:	JPM				
WSHED NAME	Primary				
SHEET FLOW: (100' MAX)					
Land Use Undeveloped			n value	% Land use	Inc n
Conc.,gravel,asphalt,bare soil			0.015	0	0
Grass Short Prairie			0.15	0	0
Maintained Grass			0.03	0	0
Woods Light Underbrush			0.4	0	0
Woods Dense underbrush			0.8	0	0
based on information for imperviousness from Corps of Engineers					
Land Use	% Conc	% Grass	n value	% Land Use	Inc n
Low D. Residential (1+ Acres)	25	75	0.21375	0	0
Med. D. Residential (1/3 Acres)	41	59	0.17135	100	0.17135
High D. Residential (1/4 Acres)	47	53	0.15545	0	0
Multifamily	70	30	0.0945	0	0
Mobile Home Parks	20	80	0.227	0	0
C.B.D.	95	5	0.02825	0	0
Strip Commercial	90	10	0.0415	0	0
Shopping Center	95	5	0.02825	0	0
Institutional-Schools	40	60	0.174	0	0
Industrial	90	10	0.0415	0	0
Highway ROW	35	65	0.18725	0	0
Public Utilities	60	40	0.121	0	0
Vacant urban land and	6	84	0.2361	0	0
Parks	0	0	0	0	0
Other	0	0	0	0	0
TOTAL			100	0.17135	
LENGTH	100 FT.	MAX 100'			
2 YR. 24 HOUR PRECIP	4.31 IN.				
SLOPE	0.010 FT/FT				
SHALLOW CONCENTRATED FLOW					
1=PAVED 2=UNPAVED	1				
LENGTH	2362.91 FT				
SLOPE	0.006 FT/FT				
COMPUTED VELOCITY FROM FIGURE 3.1=	1.508				
CHANNEL FLOW					
XSECT AREA=	100.000 SQ FT	TOPWIDTH	35		
		BOTTOM	5		
		DEPTH	5		
WETTED PERIMETER	36.623 FT				
SLOPE	0.008 FT/FT				
MANNINGS N	0.06				
COMPUTED VELOCITY	4.258 FT/S				
LENGTH	9995.43 FT				
Conditions	Adjusted	NRCS Method	Selected		
WATERSHED NUMBER	Primary	Tc (Min)	Tc (Min)	Tc (Min)	
SHEET FLOW	Max 30 Min	30.0	12.39	12.39	
SHALLOW CONCENTRATED FLOW			26.12	26.12	
CHANNEL FLOW			39.12	39.12	
TOTAL			77.63	77.63	$T_c = T_1 + T_2 + T_3 + T_4 + T_5 + T_6$
			Lag (Hrs) =	0.78	

Lag(min) = 46.58

BASIN LAG TIME CALCULATION					
USING NRCS TR55 METHOD TO COMPUTE TIME OF CONCENTRATION					
Existing Conditions					
Project Data:	Comments:				
PROJECT	AEP10412				
LOCATION	Welsh Power Plant				
DATE	Dec-10				
BASIN COND.					
BY:	JPM				
WSHED NAME	Secondary				
SHEET FLOW: (100' MAX)					
Land Use Undeveloped			n value	% Land use	Inc n
Conc.,gravel,asphalt,bare soil			0.015	0	0
Grass Short Prairie			0.15	100	0.15
Maintained Grass			0.03	0	0
Woods Light Underbrush			0.4	0	0
Woods Dense underbrush			0.8	0	0
	TOTAL			100	0.15
LENGTH	100	FT.	MAX 100'		
2 YR. 24 HOUR PRECIP	4.31	IN.			
SLOPE	0.020	FT/FT			
SHALLOW CONCENTRATED FLOW					
1=PAVED 2=UNPAVED	2				
LENGTH	1159.16	FT			
SLOPE	0.017	FT/FT			
COMPUTED VELOCITY FROM FIGURE 3.1=	2.119				
WATERSHED NUMBER	Secondary	Conditions	Adjusted Tc (Min)	NRCS Method Tc (Min)	Selected Tc (Min)
SHEET FLOW	Max 30 Min		30.0	8.44	8.44
SHALLOW CONCENTRATED FLOW				9.12	9.12
TOTAL				17.56	17.56
				Lag (Hrs) =	0.18
				Lag(min) =	10.53

$$T_1 = 0.007 \times \frac{(n \times L)^{0.8}}{R^{0.5} \times S^{0.4}}$$

$$T_2 = \frac{L}{60 \times V}$$

$$T_c = T_1 + T_2 + T_3 + T_4 + T_5 + T_6$$

Basin	Curve Number (AMC II)	Area_acre
Primary	76.6	1127.5
Secondary	74.9	39.9

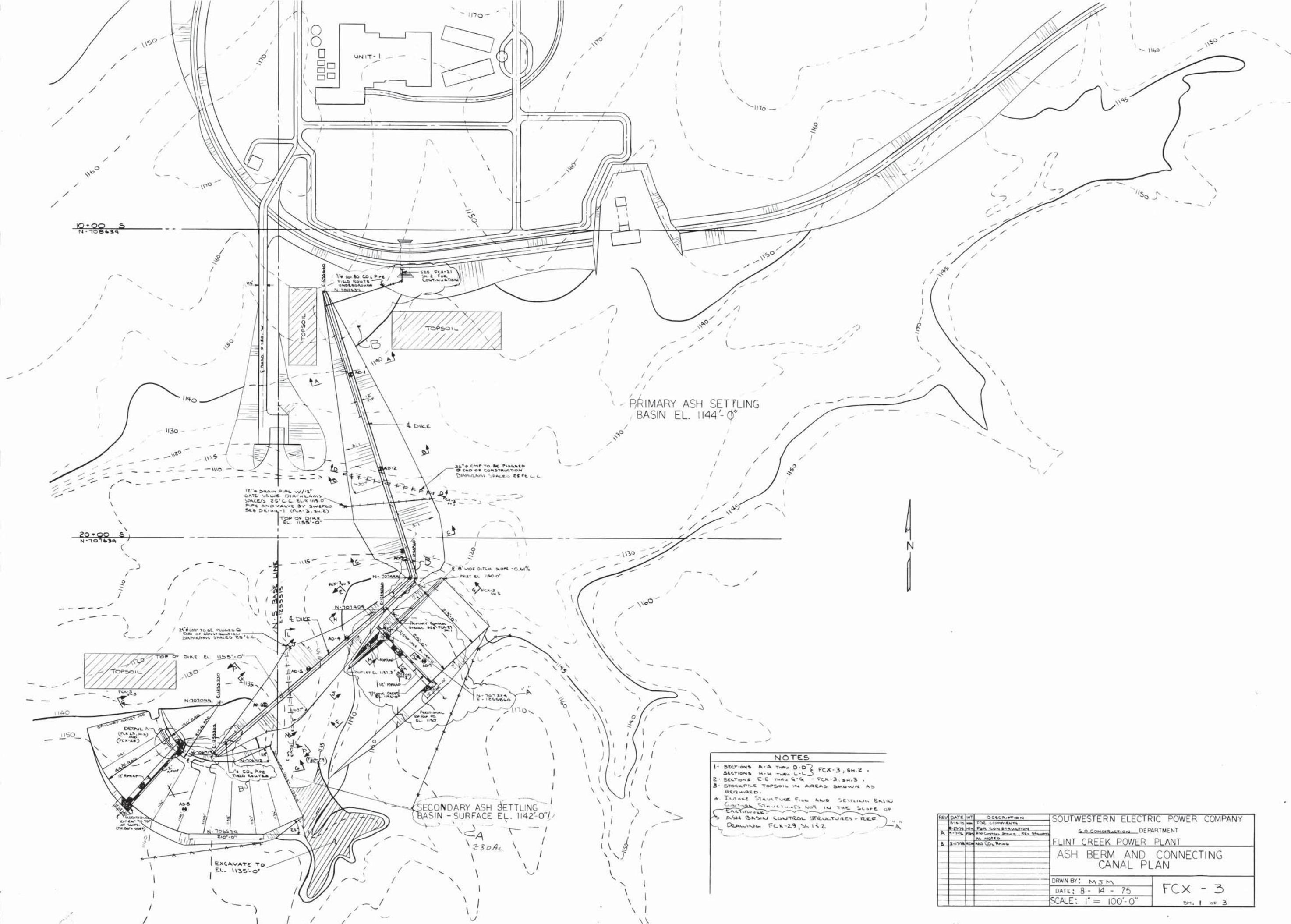
Name	GRIDCODE	HSG	Area_ft^2	Area_acre	CN	Inc. CN
Primary	11	W	492113.109	11.297	100	1.002
Primary	23	W	6432.202	0.148	100	0.013
Primary	31	W	196628.865	4.514	100	0.400
Primary	41	W	151664.101	3.482	100	0.309
Primary	71	W	22148.379	0.508	100	0.045
Primary	81	W	58928.772	1.353	100	0.120
Primary	11	B	219057.220	5.029	100	0.446
Primary	21	B	412397.879	9.467	79	0.663
Primary	22	B	511234.844	11.736	68	0.708
Primary	23	B	656437.501	15.070	85	1.136
Primary	31	B	336825.792	7.732	86	0.590
Primary	41	B	1432215.235	32.879	60	1.750
Primary	71	B	192038.278	4.409	61	0.239
Primary	81	B	3304498.256	75.861	61	4.104
Primary	90	B	53877.802	1.237	66	0.072
Primary	11	C	10933.953	0.251	100	0.022
Primary	21	C	2381557.518	54.673	86	4.170
Primary	22	C	564650.628	12.963	79	0.908
Primary	23	C	173626.908	3.986	90	0.318
Primary	31	C	185483.243	4.258	91	0.344
Primary	41	C	1450486.842	33.299	73	2.156
Primary	71	C	335571.928	7.704	74	0.506
Primary	81	C	24105757.161	553.392	74	36.321
Primary	90	C	4267.451	0.098	77	0.007
Primary	21	D	1245218.812	28.586	89	2.257
Primary	22	D	1727655.376	39.662	84	2.955
Primary	23	D	164748.228	3.782	92	0.309
Primary	31	D	1950.419	0.045	94	0.004
Primary	41	D	733098.073	16.830	79	1.179
Primary	81	D	5876087.739	134.896	80	9.572
Primary	24	B	1488256.200	34.166	92	2.788
Primary	24	C	306439.480	7.035	94	0.587
Primary	24	C	310317.021	7.124	94	0.594
Secondary	11	B	60082.739	1.379	100	3.457
Secondary	41	B	192385.993	4.417	60	6.642

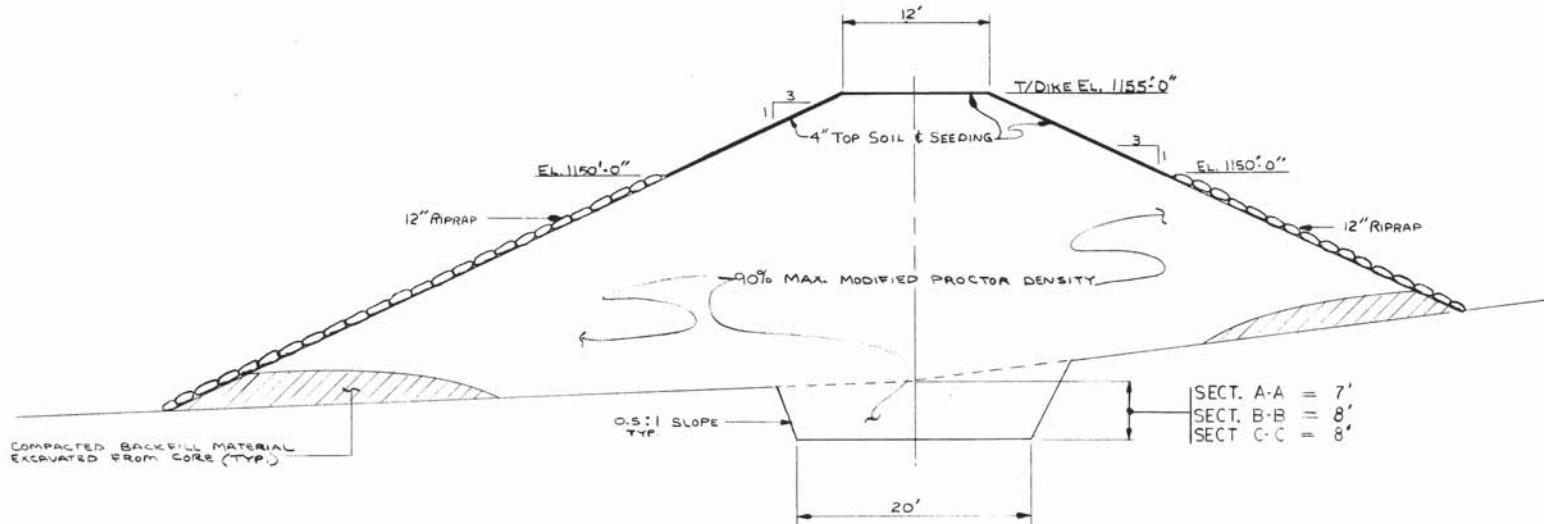
Basin	Area_acre
Primary	1127.47
Secondary	39.90

Name	GRIDCODE	HSG	Area_ft^2	Area_acre	CN	Inc. CN
Secondary	81	B	90951.376	2.088	61	3.192
Secondary	11	C	106317.260	2.441	100	6.117
Secondary	21	C	107888.311	2.477	86	5.339
Secondary	41	C	210068.421	4.823	73	8.824
Secondary	71	C	33928.559	0.779	74	1.445
Secondary	81	C	936337.296	21.495	74	39.868

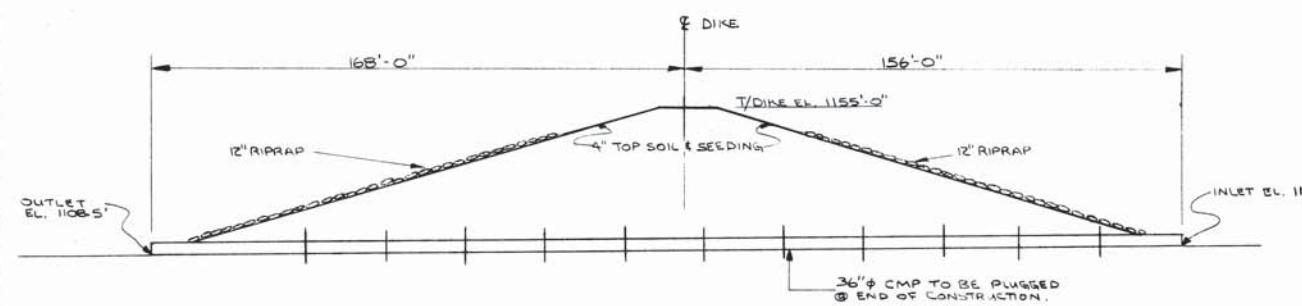
GRIDCODE	NLCD Description	TR-55 Description	Curve Number						
			A	B	B/C	C	C/D	D	W
11	Open Water	Water	100	100	100	100	100	100	100
21	Developed, Open Space	Open Space - Poor	68	79	83	86	88	89	100
22	Developed, Low Intensity	Low Density Residential acre	51	68	74	79	82	84	100
23	Developed, Medium Intensity	High Density Residential	77	85	88	90	91	92	100
24	Developed, High Intensity	Commercial	89	92	93	94	95	95	100
31	Barren Land	Fallow - Bare	77	86	89	91	93	94	100
41	Deciduous Forest	Woods - Fair	36	60	67	73	76	79	100
42	Evergreen Forest	Woods - Fair	36	60	67	73	76	79	100
43	Mixed Forest	Woods - Fair	36	60	67	73	76	79	100
52	Scrub/Shrub	Brush - Fair	35	56	63	70	74	77	100
71	Grassland/Herbaceous	Open Space - Good	39	61	68	74	77	80	100
81	Pasture/Hay	Open Space - Good	39	61	68	74	77	80	100
82	Cultivated Crops	Row Crops SR - Good	67	78	82	85	87	89	100
90	Woody Wetlands	Woods - Poor	45	66	72	77	80	83	100

Appendix C Pertinent Drawings

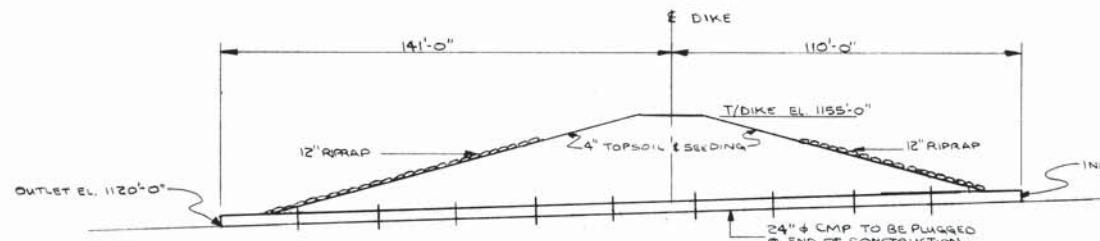




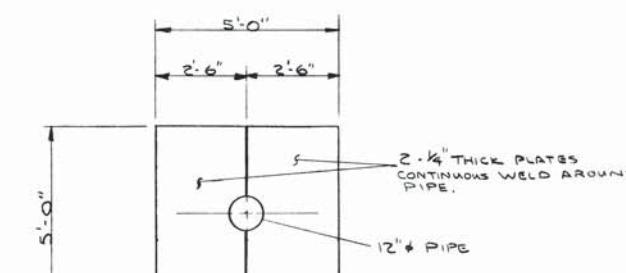
SECTION "A-A" THRU "C-C"
PRIMARY ASH POND DIKE



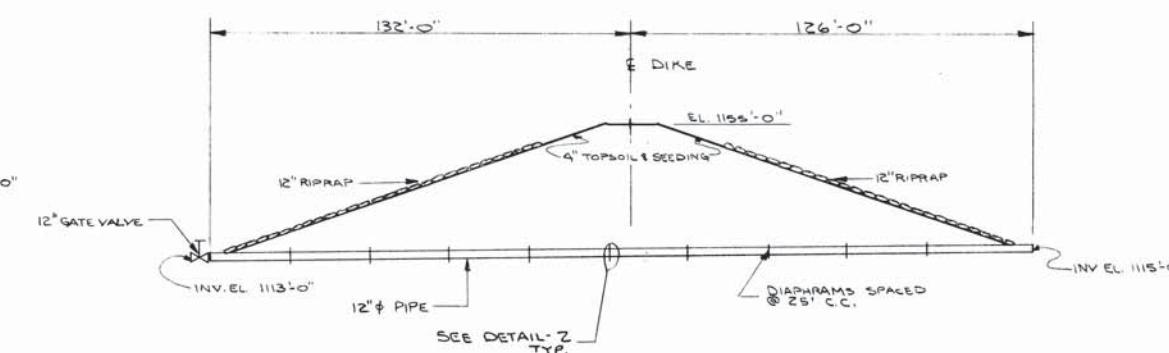
SECTION "D-D"
N.T.S.



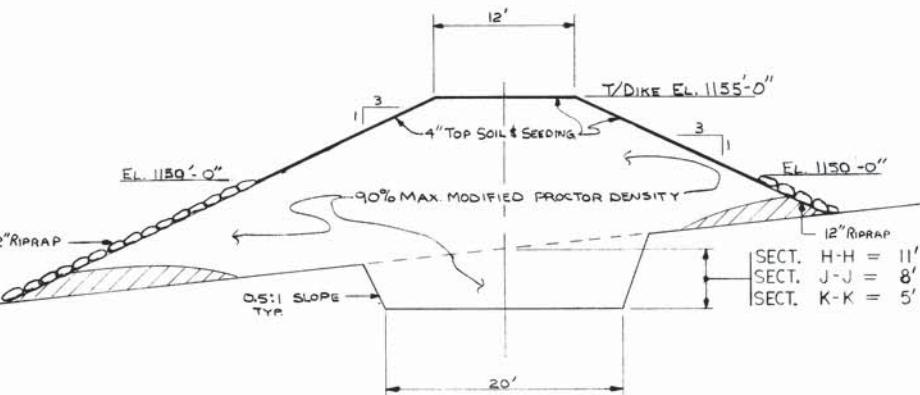
SECTION "L-L"
N.T.S.



MATERIAL BY SWEPCO.
DETAIL 2
TYPICAL DIAPHRAM



DETAIL -1



SECTION "H-H" THRU "K-K"
SECONDARY ASH POND DIKE

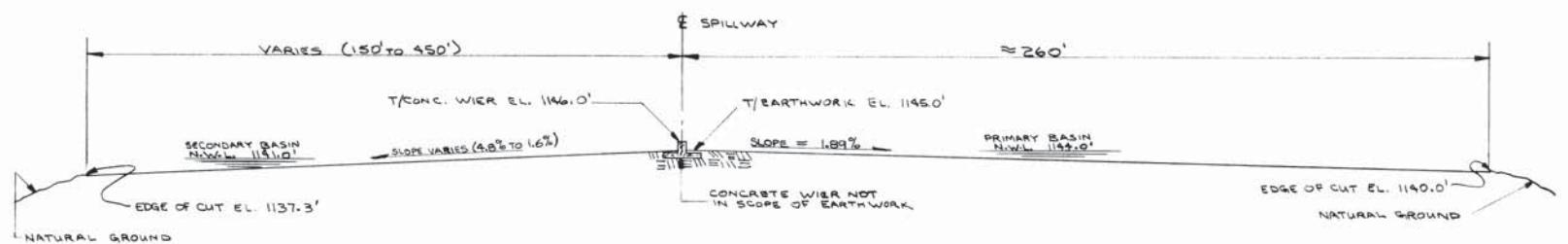
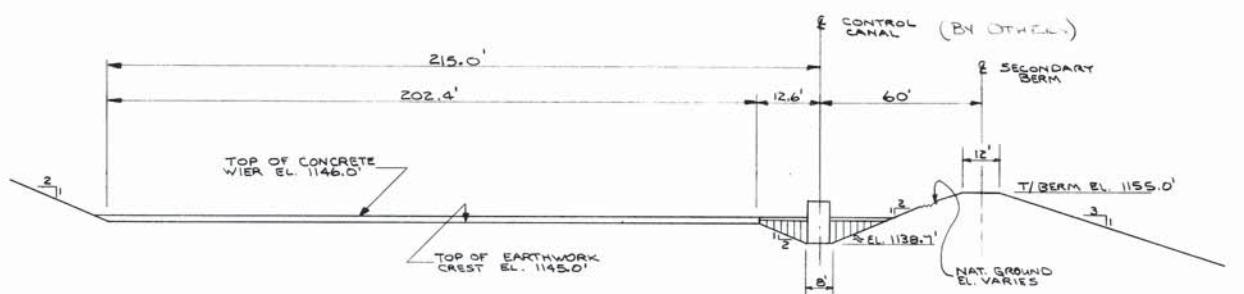
NOTES

- CONTRACTOR SHALL STRIP TOPSOIL FROM DIKE AND SPILLWAY AREA AND STOCKPILE AS SHOWN ON FCX-3, SH. 1.
- EXCAVATED MATERIAL FROM CORE TO BE USED AS COMPACTED FILL IN TOE OF BERMS.
- CORE TO BE CUT TO DESIGN SECTION; DEPTH OF CORE SHALL VARY BETWEEN SECTIONS. UPON COMPLETION OF CORE EXCAVATION, OWNER'S ENGINEER SHALL MAKE INSPECTION BEFORE BACKFILL BEGINS.
- CONTRACTOR SHALL PROVIDE DRAINAGE OF ASH BASIN AREA DURING CONSTRUCTION; ASH BASIN CONNECTING CANAL AND SPILLWAY SHALL BE CUT TO ALLOW FOR DRAINAGE BEFORE BACKFILL BEGINS.
- CONTRACTOR TO FURNISH (1) - 36" #1 AND (1) - 24" #6 GA. CMP AS SHOWN WITH DIAPHRAMS SPACED AT 25' INTERVALS. ACTUAL CULVERT INVERT ELEVATIONS WILL VARY DUE TO FIELD CONDITIONS.
- 12" PIPE AND 12" GATE VALVE FURNISHED BY SWEPCO.

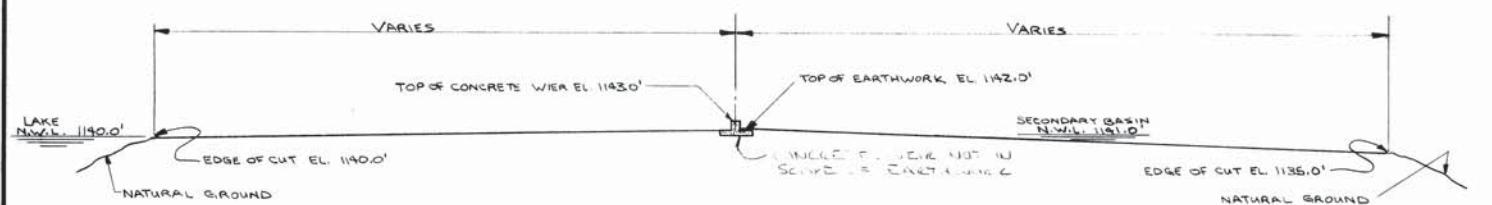
7/12/74	FOR BID
Rev. 15 MIM	FOR CONSTRUCTION
B-17-78	SUBJECT
ASH POND DIKE	
CROSS - SECTIONS	
FLINT CREEK POWER PLANT	
SOUTHWESTERN ELECTRIC POWER CO.	
G.O. CONSTRUCTION DEPARTMENT	
DIVISION	
APPROVED:	ENGR. IN CHARGE
APPROVED:	DIV. SUPT.
APPROVED:	CHIEF ENGR.
DRWN. BY MJM	WORK ORDER
TRAC. BY	
DATE: 7-10-74	DRW. NO. FCX - 3
SCALE: NONE	

SHEET

FCX - 3
SH. 2

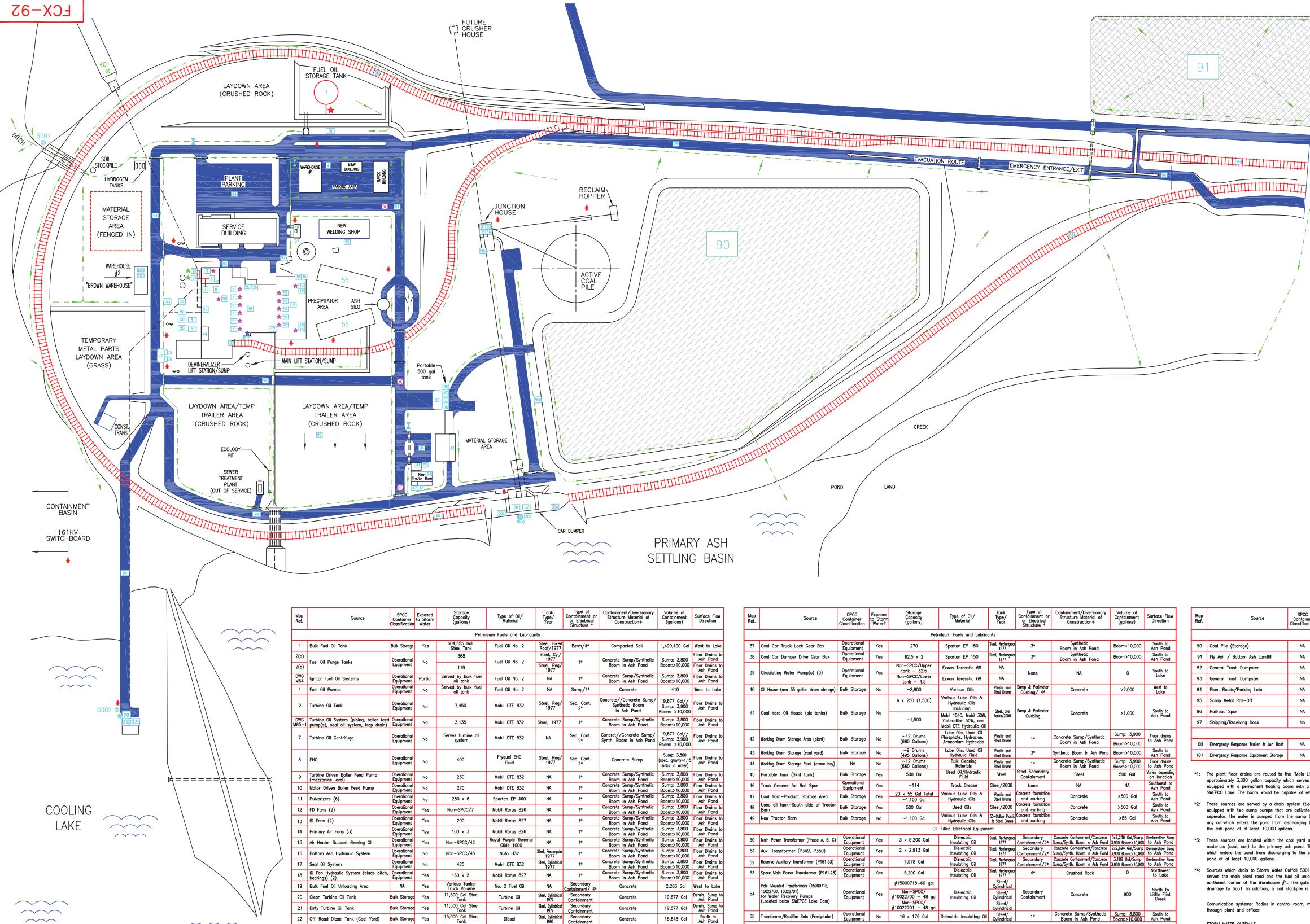


SECTION "F-F"



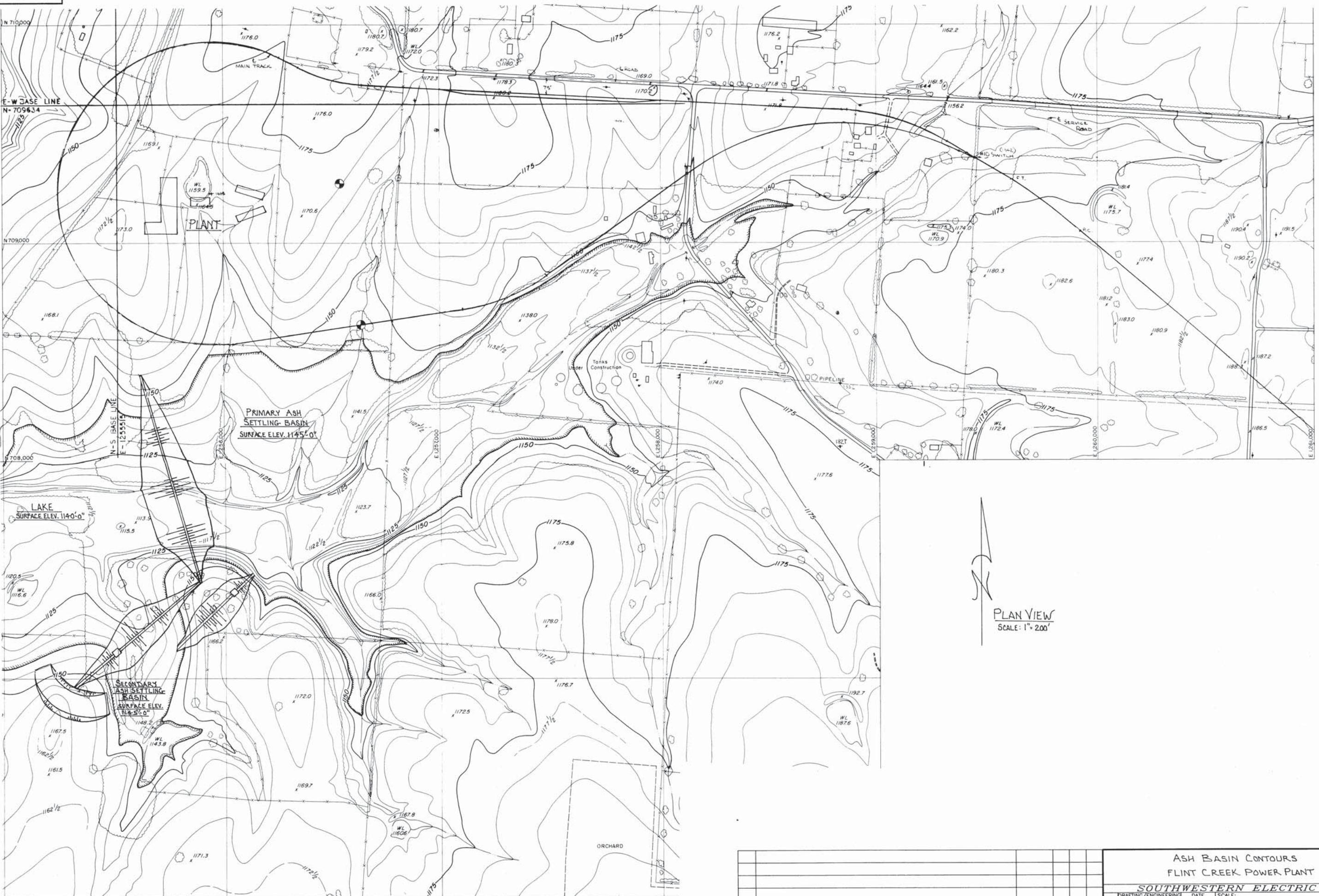
SECTION "G-G"

B-2975	W.M.M.	FOR CONSTRUCTION
B 3.17-78		
REV. DATE	BY	SUBJECT
ASH POND SPILLWAY SECTIONS		
FLINT CREEK POWER PLANT		
SOUTHWESTERN ELECTRIC POWER CO.		
G.O. CONSTRUCTION DEPARTMENT		
DIVISION		
APPROVED	ENGR. IN CHARGE	
APPROVED	DIV. SUPT.	
APPROVED	CHIEF ENGR.	
DRAWN BY	W.M.M.	WORK ORDER
TRAC. BY		
DATE:	B - 18 - 75	DRAW. NO.
SCALE:	1" = 30'	FCX - 3
SHEET	SH. 3	



Map Ref.	Source	SPCC Container Classification	Exposed to Storm Water?	Storage Capacity (gallons)	Type of Oil/ Material	Task/Type/ Year	Type of Containment or Electrical Structure	Containment/Diversionary Structure Material/ Construction+	Volume of Containment (gallons)	Surface Flow Direction
Petroleum Fuels and Lubricants										
1	Bulk Fuel Oil Tank	Bulk Storage	Yes	604,555 Gal	Fuel Oil No. 2	Steel, Fixed	Berm/*	Compacted Soil	1,499,400 Gal	West to Lake
2(g)	Fuel Oil Purge Tanks	Operational Equipment	No	368	Fuel Oil No. 2	Steel, Reg'd 1977	1*	Concrete Sump/Synthetic Boom in Ash Pond	3,800	West to Lake
2(b)	DWG M64 Ignitor Fuel Oil Systems	Operational Equipment	Partial	119	Fuel Oil No. 2	Steel, Reg'd 1977	1*	Concrete Sump/Synthetic Boom in Ash Pond	3,800	West to Lake
4	Fuel Oil Pumps	Operational Equipment	No	Served by bulk fuel oil tank	Fuel Oil No. 2	NA	Sump/*	Concrete	410	West to Lake
5	Turbine Oil Tank	Operational Equipment	No	7,450	Mobil DTE 832	Steel, Reg'd 1977	Sec. Cont. 2*	Concrete/Concrete Sump/ Synthetic Boom in Ash Pond	19,677 Gal// Sump: 3,900	West to Lake
DWG Turbine Oil System (piping, boiler feed M55-1 pump(s), seal oil systems, trap drain)	Operational Equipment	No	3,135	Mobil DTE 832	Steel, 1977	1*	Concrete Sump/Synthetic Boom in Ash Pond	3,800	West to Lake	
7	Turbine Oil Centrifuge	Operational Equipment	No	Serves turbine oil system	Mobil DTE 832	NA	Sec. Cont. 2*	Concrete/Concrete Sump/ Synthetic Boom in Ash Pond	19,677 Gal// Sump: 3,900	West to Lake
8	ENC	Operational Equipment	No	400	Fryquel EH Fluid	Steel, Reg'd 1977	Sec. Cont. 2*	Concrete Sump	3,800	West to Lake
9	Turbine Driven Boiler Feed Pump	Operational Equipment	No	230	Mobil DTE 832	NA	1*	Concrete Sump/Synthetic Boom in Ash Pond	3,800	West to Lake
10	Motor Driven Boiler Feed Pump	Operational Equipment	No	270	Mobil DTE 832	NA	1*	Concrete Sump/Synthetic Boom in Ash Pond	3,800	West to Lake
11	Pulverizer (6)	Operational Equipment	No	250 x 6	Spartan EP 460	NA	1*	Concrete Sump/Synthetic Boom in Ash Pond	3,800	West to Lake
12	FD Fans (2)	Operational Equipment	Yes	Non-SPCC/7	Mobil Rorus 826	NA	1*	Concrete Sump/Synthetic Boom in Ash Pond	3,800	West to Lake
13	ID Fans (2)	Operational Equipment	Yes	200	Mobil Rorus 827	NA	1*	Concrete Sump/Synthetic Boom in Ash Pond	3,800	West to Lake
14	Primary Air Fans (3)	Operational Equipment	Yes	100 x 3	Mobil Rorus 826	NA	1*	Concrete Sump/Synthetic Boom in Ash Pond	3,800	West to Lake
15	Air Heater Support Bearing Oil	Operational Equipment	No	Non-SPCC/4	Royal Purple Thermal Grade 1000	NA	1*	Concrete Sump/Synthetic Boom in Ash Pond	3,800	West to Lake
16	Bottom Ash Hydraulic System	Operational Equipment	No	Non-SPCC/45	Nuto H32	Steel, Rectangular	1*	Concrete Sump/Synthetic Boom in Ash Pond	3,800	West to Lake
17	Seal Oil System	Operational Equipment	No	425	Mobil DTE 832	Steel, Rectangular	1*	Concrete Sump/Synthetic Boom in Ash Pond	3,800	West to Lake
18	ID Fan Hydraulic System (blade pitch bearings) (2)	Operational Equipment	Yes	160 x 2	Mobil Rorus 827	NA	1*	Concrete Sump/Synthetic Boom in Ash Pond	3,800	West to Lake
19	Bulk Fuel Oil Unloading Area	Bulk Storage	NA	Various Tanker Truck Volume	No. 2 Fuel Oil	NA	Secondary Containment/*	Concrete	2,283 Gal	West to Lake
20	Clean Oilline Oil Tank	Bulk Storage	NA	11,000 Gal Steel	Turbine Oil	Steel, Rectangular	Secondary Containment	Concrete	19,677 Gal	West to Lake
21	Dirty Turbine Oil Tank	Bulk Storage	Yes	11,000 Gal Steel	Turbine Oil	Steel, Rectangular	Secondary Containment	Concrete	19,677 Gal	West to Lake
22	Off-Road Diesel Tank (Coal Yard)	Bulk Storage	Yes	15,000 Gal Steel Tank	Diesel	Steel, Cylindrical	Secondary Containment	Concrete	15,648 Gal	South to Ash Pond
23	Unleaded Gasoline Tank (Coal Yard)	Bulk Storage	Yes	1,500 Gal Steel Tank	Gasoline	Steel, Rectangular	Secondary Containment	Concrete	15,648 Gal	South to Ash Pond
24	Kerosene Tank (Coal Yard)	Bulk Storage	Yes	560 Gal Steel Tank	Kerosene	Steel, Rectangular	Secondary Containment	Concrete	15,648 Gal	South to Ash Pond
25	Desert/Kerosene/Gasoline Unloading Area (Coal Yard)	NA	Yes	Various Tanker Truck Volume	Desert/Kerosene/ Gasoline	Steel, Rectangular	Drop-off Box with Valve	Concrete	0/5,000 Gal	South to Ash Pond
26	Used Oil Tank (Coal Yard)	Bulk Storage	Yes	1,000 Gal Steel Tank	Used Oil	Steel, Rectangular	Secondary Containment	Concrete	15,648 Gal	South to Ash Pond
27	Used Oil Tank (Oil House)	Bulk Storage	No	500 Gal Steel Tank	Used Oil	Steel, Rectangular	Secondary Containment/*	Concrete	1,588 Gal	West to Lake
28	Fire Pump Diesel Tank (Intake Structure)	Bulk Storage	Yes	200 Gal Steel Tank	Diesel	Steel, Rectangular	Secondary Containment	Steel	601 Gal	South to Lake
29	Diesel Fuel Tank (Once/Generator)	Bulk Storage	No	Non-SPCC/25 Gal Steel Tank	Diesel	Steel, Rectangular	Secondary Containment	Steel	177 Gal	South to Ash Pond
30a	Hydraulic Fluid Tank (Coal Car Positioning System)	Operational Equipment	Yes	230 Gal Steel Tank	Hydraulic Fluid	Steel, Rectangular	Secondary Containment	Concrete	1,497 Gal	South to Ash Pond
30b	Hydraulic Fluid Tank (Coal Car Positioning System)	Operational Equipment	No	490 Gal Steel Tank	Hydraulic Fluid	Steel, Rectangular	Secondary Containment	Concrete	703 Gal	South to Ash Pond
30c	Hydraulic Fluid Tank (Coal Car Positioning System)	Operational Equipment	Yes	230 Gal Steel Tank	Hydraulic Fluid	Steel, Rectangular	Secondary Containment	Concrete	1,690 Gal	South to Ash Pond
31	Cool Conveyor Belt Motors	Operational Equipment	Yes	Non-SPCC/45	Sparton EP 150	Steel, Rectangular	3*	Booms in Ash Pond	10,000 Gal	West to Lake
32	Cool Conveyor Belt Gear Box - A Bell	Operational Equipment	Yes	86	Sparton EP 150	Steel, Rectangular	3*	Booms in Ash Pond	>10,000 Gal	South to Ash Pond
33	Cool Conveyor Belt Gear Box - B Bell	Operational Equipment	Yes	52	Sparton EP 150	Steel, Rectangular	3*	Booms in Ash Pond	>10,000 Gal	South to Ash Pond
34	Cool Conveyor Belt Gear Box - C Bell	Operational Equipment	Yes	Non-SPCC/30	Sparton EP 150	Steel, Rectangular	3*	Booms in Ash Pond	>10,000 Gal	South to Ash Pond
35	Cool Conveyor Belt Gear Box - D Bell	Operational Equipment	No	52 x 2	Sparton EP 150	Steel, Rectangular	2*	Booms in Ash Pond	10,000 Gal	South to Ash Pond
36	Cool Conveyor Belt Gear Box - E Bell	Operational Equipment	No	Non-SPCC/5.5 x 2	Sparton EP 150	Steel, Rectangular	2*	Concrete Sump/Synthetic Boom	3,900 Gal	South to Ash Pond

Map Ref.	Source	SPCC Container Classification	Exposed to Storm Water?	Storage Capacity (gallons)	Type of Oil/ Material	Task/Type/ Year	Type of Containment or Electrical Structure	Containment/Diversionary Structure Material/ Construction+	Volume of Containment (gallons)	Surface Flow Direction
Petroleum Fuels and Lubricants										
37	Coal Car Truck Lock Gear Box	Operational Equipment	Yes	270	Sparton EP 150	Steel, Rectangular	3*	Synthetic Boom in Ash Pond	3,000 Gal	South to Ash Pond
38	Coal Car Dumper Drive Gear Box	Operational Equipment	Yes	625 & 2	Sparton EP 150	Steel, Rectangular	3*	Synthetic Boom in Ash Pond	>10,000 Gal	South to Ash Pond
39	Circulating Water Pump(s) (3)	Operational Equipment	Yes	Non-SPCC/Upper tank: 32 Gal	Exxon Teresian 68	NA	None	NA	0	South to Lake
40	Oil House (new 55 gallon drum storage)	Bulk Storage	No	~2,800	Various Oils	Plastic and Steel Drums	3*	Concrete Foundation and curbing	>2,000 Gal	West to Lake
41	Cool Yard Oil House (six tanks)	Bulk Storage	No	6 x 250 (1,500)	Various Lubric Oils & Greases	Plastic and Steel Drums	3*	Concrete Foundation and curbing	>1,000 Gal	South to Ash Pond
42	Working Drum Storage Area (plant)	Bulk Storage	No	~1,500	Drums	Plastic and Steel Drums	3*	Concrete Sump/Synthetic Boom in Ash Pond	3,900 Gal	West to Lake
43	Working Drum Storage (coal yard)	Bulk Storage	No	~495 (Galons)	Lube Oils, Used Oil Hydraulic Fluid	Plastic and Steel Drums	3*	Concrete Sump/Synthetic Boom in Ash Pond	10,000 Gal	South to Ash Pond
44	Working Drum Storage Rock (crane bay)	NA	No	~660 (Galons)	Hydraulic Fluid	Plastic and Steel Drums	3*	Concrete Sump/Synthetic Boom in Ash Pond	3,900 Gal	South to Ash Pond
45	Portable Tank (Slid Tank)	Bulk Storage	Yes	500 Gal	Used/hydraulic	Steel	Secondary Containment	Steel	500 Gal	Varies depending on location
46	Track Greaser for Rail Spur	Operational Equipment	Yes	~114	Track Grease	Steel	2*	Concrete foundation and curb	NA	NA
47	Cool Yard-Product Storage Area	Bulk Storage	No	20 x 55 Gal Total	Used Oil/hyd. Oils & hydraulic Oils	Plastic and Steel Drums	3*	Concrete foundation and curb	>50 Gal	South to Ash Pond
48	Used oil tank-South side of Tractor Barn	Bulk Storage	Yes	500 Gal	Used Oil	Steel	2*	Concrete foundation and curb	>50 Gal	South to Ash Pond
49	New Tractor Barn	Bulk Storage	No	~1,100 Gal	Various Lubric Oils & hydraulic Oils	Plastic and Steel Drums	3*	Concrete foundation and curbing	>55 Gal	South to Ash Pond
Oil-Filled Electrical Equipment										
50	Main Power Transformer (Phase A, B, C)	Operational Equipment	Yes	3 x 5,200 Gal	Dielectric Insulating Oil	Steel, Rectangular	Secondary Containment/*	Concrete Sump/Synthetic Boom in Ash Pond	10,000 Gal/Stormwater	South to Ash Pond
51	Aux. Transformer (P349, P350)	Operational Equipment	Yes	2 x 2,913 Gal	Dielectric Insulating Oil	Steel, Rectangular	Secondary Containment/*	Concrete Sump/Synthetic Boom in Ash Pond	5,826 Gal/Stormwater	South to Ash Pond
52	Reserve Auxiliary Transformer (P161.33)	Operational Equipment	Yes	7,578 Gal	Dielectric Insulating Oil	Steel, Rectangular	Secondary Containment/*	Concrete Sump/Synthetic Boom in Ash Pond	15,156 Gal/Stormwater	South to Ash Pond
53	Spore Main Power Transformer (P161.23)	Operational Equipment	Yes	5,200 Gal	Dielectric Insulating Oil	Steel, Cylindrical	4*	Crushed Rock	0	Northwest to Lake
54	Pole-Mounted Transformers (1000/16, 100/230, 1022/210)	Operational Equipment	Yes	#1500/16-60 gal	Non-SPCC/100/230-48 gal	Dielectric Insulating Oil	Steel, Cylindrical	Steel	900 Gal	North to Little Pine Creek
55	Transformer/Rectifier Sets (Precipitator)	Operational Equipment	No	16 x 176 Gal	Dielectric Insulating Oil	Steel/Cylindrical	1*	Concrete Sump/Synthetic Boom in Ash Pond	3,800 Gal	South to Ash Pond
Above Ground Piping										
60	Fuel Oil Piping (Ref. Deg. M64)	NA	Partial	NA	Fuel Oil No. 2	NA	1*/4*	Concrete Sump/Synthetic Boom in Ash Pond	3,800 Gal	But to Lake/Stormwater
61	Turbine Oil Piping (Ref. Deg. M5. sh.1)	NA	Partial	NA	Mobil DTE 832	NA	1*	Concrete Sump/Synthetic Boom in Ash Pond	3,800 Gal	But to Lake/Stormwater



ASH BASIN CONTOURS FLINT CREEK POWER PLANT			
SOUTHWESTERN ELECTRIC POWER CO.			
DRAFTING/ENGINEERING	DATE	SCALE	DWG. NO.
DR: <i>Bill Park</i>	11-18-05	BP - -	FCX-96
ENG: -	-	-	REV. No. -
APP: -	-	-	SHT. 1 of 1

RELEASED FOR INFORMATION

